

# TRAFFIC AND PARKING IMPACT ASSESSMENT OF THE PROPOSED RESIDENTIAL TOWNHOUSES AT 262 ABERGLASSLYN ROAD, ABERGLASSLYN



Address: Shop 7, 720 Old Princes Highway Sutherland NSW 2232
Postal: P.O Box 66 Sutherland NSW 1499

Telephone: +61 2 9521 7199
Web: www.mclarentraffic.com.au
Email: admin@mclarentraffic.com.au

Division of RAMTRANS Australia ABN: 45067491678 RPEQ: 19457

Transport Planning, Traffic Impact Assessments, Road Safety Audits, Expert Witness



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Site Address: 262 Aberglasslyn Road, Aberglasslyn

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## 1 INTRODUCTION

*M<sup>c</sup>Laren Traffic Engineering* was commissioned by *MHM Construction* Group to provide a traffic and parking impact assessment of the proposed residential townhouses at 262 Aberglasslyn Road, Aberglasslyn as depicted in **Annexure A**.

## 1.1 Description and Scale of Development

The proposed development has the following characteristics relevant to traffic and parking:

- A total of 24 townhouse dwellings consisting of;
  - 18 two-bedroom units;
  - 6 three-bedroom units.
- A total of 35 car parking spaces provided on-site including:
  - 29 residential car parking spaces within enclosed garages
  - 6 visitor car parking spaces.

## 1.2 State Environmental Planning Policy (Transport and Infrastructure) 2021

The proposed development does not qualify as a traffic generating development with relevant size and/or capacity under *Clause 2.122* of the *SEPP (Transport and Infrastructure) 2021*. Accordingly, formal referral to Transport for NSW (TfNSW) is unnecessary and the application can be assessed by Maitland Council officers accordingly.

## 1.3 Site Description

The subject development is currently zoned R1 – General Residential under the Maitland Council LEP 2011 and is currently unoccupied. The site has a singular frontage to Aberglasslyn Road to the west.

The site is generally surrounded by residential dwellings in all directions, with large lot residential properties primarily on the western side of Aberglasslyn Road. Woolworths Aberglasslyn located approximately 600m to the south and McKeachies Sportsground located approximately 800m to the east of site.



## 1.4 Site Context

The location of the site is shown on an aerial photo and a street map in **Figure 1** and **Figure 2** respectively.



Site Location

FIGURE 1: SITE CONTEXT - AERIAL PHOTO



Site Location

FIGURE 2: SITE CONTEXT - STREET MAP



## 2 EXISTING TRAFFIC AND PARKING CONDITIONS

## 2.1 Road Hierarchy

The road network servicing the site has characteristics as described in the following subsections.

## 2.1.1 Aberglasslyn Road

- Unclassified COLLECTOR Road;
- Approximately 8m wide two-way carriageway facilitating one (1) traffic flow lane in each direction;
- Signposted 60km/h speed limit;
- No formal kerbside parking permitted along both sides of the road.

## 2.1.2 Warbler Avenue

- Unclassified COLLECTOR Road;
- Approximately 11m wide two-way carriageway facilitating one (1) traffic flow lane in each direction and kerbside parking;
- Default 50km/h speed limit applies;
- Generally, unrestricted kerbside parking permitted along both sides of the road.

## 2.2 Existing Traffic Management

- Roundabout controlled intersection of Aberglasslyn Road / Warbler Avenue / Tea
   Tree Avenue;
- Roundabout controlled intersection of Aberglasslyn Road / Denton Park Drive / McKeachie Drive;
- "STOP" controlled intersection of Aberglasslyn Road / Oakhampton Road.



## 2.3 Existing Traffic Environment

Intersection traffic surveys were conducted at the intersections of Aberglasslyn Road / Warbler Avenue / Tea Tree Avenue and Aberglasslyn Road / Denton Park Drive / McKeachie Drive from 7:00am to 9:30am and 2:30pm to 6:00pm on Thursday 3 November 2022 representing a typical operating weekday. The full survey results are shown in **Annexure B** for reference.

## 2.3.1 Existing Road Performance

The performance of the surrounding intersections under the existing traffic conditions has been assessed using SIDRA INTERSECTION 9.1, **Table 1** summarises the resultant intersection performance data, with full SIDRA results reproduced in **Annexure C**.

TABLE 1: EXISTING INTERSECTION PERFORMANCES (SIDRA INTERSECTION 9.1)

Intersection	Peak Hour	Degree of Saturation <sup>(1)</sup>	Average Delay <sup>(2)</sup> (sec/veh)	Level of Service <sup>(3)(4)</sup>	Control Type	Worst Movement
			EXISTING PERFOR	MANCE		
		0.07	5.7	Α		UT from
Tea Tree Avenue	AM	0.07	(Worst: 12.1)	(Worst: B)	Davisdahavit	Aberglasslyn Road
/ Warbler Avenue	DM	PM 0.11	5.7	Α	Roundabout	UT from
	PIVI		(Worst: 10.7)	(Worst: B)		Aberglasslyn Road
	0.04	0.38	6.2	Α		UT from
McKeachie Drive /	AM	VI 0.36	(Worst: 11.7)	(Worst: B)	Roundabout	Aberglasslyn Road
Denton Park Drive	DM	PM 0.45	6.9	Α	Roundabout	UT from
	PIVI	0.45	(Worst: 12.1)	(Worst: B)		Aberglasslyn Road
	0.04	0.05	0.1	NA		RT from Site
Aberglasslyn	AIVI	AM 0.05	(Worst: 6)	(Worst: A)	a	Driveway
Road / Site Driveway	DM	0.00	0.1	NA	Give Way	RT from Site
PM		0.08	(Worst: 6.2)	(Worst: A)		Driveway

### Notes:

- (1) The Degree of Saturation is the ratio of demand to capacity for the most disadvantaged movement.
- (2) The average delay is the delay experienced on average by all vehicles. The value in brackets represents the delay to the most disadvantaged movement.
- (3) The Level of Service is a qualitative measure of performance describing operational conditions. There are six levels of service, designated from A to F, with A representing the best operational condition and level of service F the worst. The LoS of the intersection is shown in bold, and the LoS of the most disadvantaged movement is shown in brackets.
- (4) No overall Level of Service is provided for Give Way and Stop controlled intersections as the low delays associated with the dominant movements skew the average delay of the intersection. The Level of Service of the worst approach is an indicator of the operation of the intersection, with a worse Level of Service corresponding to long delays and reduced safety outcomes for that approach.

As shown, the relevant intersections are currently performing at a high level of efficiency, with Level of Service "A" conditions in both the AM & PM peak hour periods. The Level of Service "A" performance is characterised by low approach delays and spare capacity.



## 2.4 Public Transport

The subject site has access to existing bus stop (ID: 2320327) located approximately 700m walking distance to the east of site on McKeachie Drive. The bus stop services existing bus route 186 (Rutherford to Aberglasslyn via Denton Park Drive (Loop Service)) provided by Hunter Valley Buses.

The location of the site subject to the surrounding public transport network is shown in **Figure 3**.

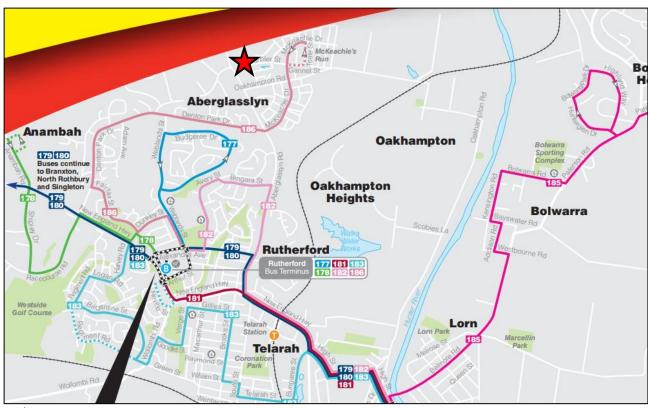




FIGURE 3: PUBLIC TRANSPORT NETWORK MAP

## 2.5 Future Road and Infrastructure Upgrades

From Maitland Council Development Application tracker and website, it appears that Aberglasslyn Road is currently undergoing upgrades to widen the carriageway and formalise kerbs and drainage along both sides of the road. The federal government has recently committed through its *Roads to Recovery Program* to an upgrade of Aberglasslyn Road north of Warbler Avenue, including rehabilitation or the road pavement and resealing, as well as widening the road to accommodate a 1m road shoulder. This work is expected to start in October 2022 and finish in May 2023.

There are no future planned road or public transport changes that will affect traffic conditions within the immediate vicinity of the subject site.



## 3 PARKING ASSESSMENT

## 3.1 Council Parking Requirement

Reference is made to the *Maitland Development Control Plan (MDCP) 2011 Part C – Design Guidelines* which designates the following parking rates applicable to the proposed development:

## 1.2 Calculation of Numbers

Where the calculation results in a fraction of a space, the total number of parking spaces required will be the next highest whole number.

## Appendix A – Car Parking Requirements for Specific Land Uses

Multi dwelling Housing / Dual Occupancy

1 space for each one- or two-bedroom dwelling

or

2 spaces for each dwelling containing more than two bedrooms

plus

1 visitor space for the first three dwellings and 1 space for every five thereafter or part thereof

**Table 2** presents the parking requirements of the proposal according to the Council's above car parking rates.

**TABLE 2: DCP PARKING RATES** 

Land Use	Туре	Scale	Rate	Spaces Required
	Two-Bedroom Unit	19	1 per dwelling	19
Multi Dwelling	Three-Bedroom Unit	5	2 per dwelling	10
Housing	Visitor	3	1 per 3 dwellings	1
	Visitor 21		1 per 5 dwellings	5 (4.2)
TOTAL	-	-	-	35

As shown, strict application of the DCP requires the provision of **35** car parking spaces, (with **29** for residential use and **6** for visitor use). The proposed plans detail the provision of **35** car parking spaces, resulting in compliance with Council's DCP parking requirements.



## 3.2 Parking for People with Disabilities

The MDCP states the following regarding accessible parking provision relevant to the proposed development:

12.4 Where an adaptable dwelling is required in accordance with the provisions of this Plan, one (1) accessible car parking space shall be provided for every adaptable dwelling. This is in addition to any accessible parking required by Section 15 of this chapter.

The proposed site requires three (3) adaptable dwellings and therefore three (3) adaptable spaces. The proposed car parking layout incorporates three (3) accessible parking spaces within the relevant enclosed garages, resulting in compliance with Council's DCP requirements.

## 3.3 Bicycle & Motorcycle Parking Requirements

The MDCP does not require the provision of bicycle / motorcycle parking. No bicycle / motorcycle parking has been provided, satisfying Council requirements.

The proposed development provides one (1) enclosed garage per dwelling, where it would be expected that residents can store their bicycles or motorcycles if required.

## 3.4 Servicing & Loading

The MDCP does not specify the requirement of service facilities made available for residential developments. Delivery / courier vehicles to the site can utilise the existing onstreet parking and / or the provided visitor car parking spaces within the car park. These types of delivery vehicles for residential developments are infrequent and typically occur outside of peak residential visitor periods which occur after 6pm on Friday and Saturday nights. The standard size of a courier vehicle is a B99 design vehicle, which can easily park within the existing on-street kerbside parking supply or within the on-site visitor parking spaces.

It is expected that waste collection will be completed by Council's waste collection service vehicles along the Aberglasslyn Road frontage.



## 3.5 Car Park Design & Compliance

The car parking layout as depicted in **Annexure A**, has been assessed to achieve the relevant clauses and objectives of *AS2890.1:2004* and *AS4199:1995*. Any variances from standards are addressed in the following subsections including required changes, if any. Swept path testing has been undertaken and are reproduced within **Annexure D** for reference.

The proposed car parking and vehicular access design achieves the following:

- 5.5m wide two-way driveway facilitating access to Aberglasslyn Road;
- Minimum 5.5m wide parking aisles;
- Compliant ramp grades not exceeding 10%;
- Minimum 5.4m long, 2.4m wide spaces for residents;
  - Enclosed single garages with a minimum internal width of 3.5m with minimum entry width of 2.7m and a minimum internal length of 5.8m;
  - Enclosed double garages with a minimum internal width of 6.0m with minimum entry width of 5.2m and a minimum internal length of 5.6m;
- Minimum 5.4m long, 2.5m wide spaces for visitors;
- Enclosed single garage with a minimum internal width of 3.9m with minimum entry width of 3.0m for adaptable units;
- Minimum headroom of 2.2m for general circulation and 2.5m headroom clearance provided over accessible and adaptable parking areas.

Whilst the plans have been assessed to comply with the relevant standards, it is usual and expected that a design certificate be required at the Construction Certificate stage to account for any changes following the development application.

## 3.5.1 Driveway / Circulation Roadway Width

Reference is made to Clause 3.3 of *AS2890.1* which states the following regarding driveway width requirements at Category 1 access driveways:

Where the circulation roadway leading from a Category 1 access driveway is 30m or longer, or sight distance from one end to the other is restricted, and the frontage road is an arterial or sub-arterial road, both the access driveway and circulation roadway for at least the first 6m from the property boundary shall be a minimum of 5.5m wide.

The proposed circulation roadway leads from a Category 1 access driveway, is approximately 59m long and the frontage road (Aberglasslyn Road) is considered to be a sub-arterial road. Considering the above, the driveway and the first 6m of circulation roadway shall be a minimum of 5.5m wide. The proposed plans detail a 5.5m width for both the site driveway and circulation roadway resulting in compliance with *AS2890.1:2004* requirements. Furthermore, swept path testing as reproduced in **Annexure D** shows that two-way passing can occur within the site boundary.



## 4 TRAFFIC ASSESSMENT

The impact of the expected traffic generation levels associated with the subject proposal is discussed in the following sub-sections.

### 4.1 Traffic Generation

Traffic generation rates for the relevant land uses are provided in the *RTA Guide to Traffic Generating Developments (2002)* and recent supplements as adopted by Transport for NSW (TfNSW) and are as follows:

## 3.3.2 Medium density residential flat building

Smaller units and flats (up to two bedrooms):

Weekday peak hour vehicle trips = 0.4-0.5 per dwelling

Larger units and town houses (three or more bedrooms):

Weekday peak hour vehicle trips = 0.5 - 0.65 per dwelling

The resulting AM and PM peak hourly traffic generation is summarised in **Table 3**.

**TABLE 3: ESTIMATED TRAFFIC GENERATION** 

Use	Туре	Scale	Peak	Generation Rate	Trips
Residential <sup>(1)</sup>	Two	19 units	AM	0.5 per dwelling	10 (2 in, 8 out)
	Bedroom		PM		10 (8 in, 2 out)
Residential	Three Bedroom	5 units	AM	0.65 per dwelling	3 (1 in, 2 out)
			PM		3 (2 in, 1 out)
Total			AM		13 (3 in, 10 out)
iolai	-	-	PM		13 (10 in, 3 out)

Notes:

As shown, the expected traffic generation associated with the proposed development is in the order of **13** vehicle trips in the AM peak period (3 in, 10 out) and **13** vehicle trips in the PM peak period (10 in, 3 out).

<sup>(1)</sup> Assumes 20% inbound & 80% outbound during AM peak. Vice versa for PM.



### 4.2 Cumulative Traffic Assessment

A cumulative traffic assessment has been undertaken and incorporates the traffic generated by the proposed multi dwelling housing developments on 266 Aberglasslyn Road and 272 Aberglasslyn Road.

The resulting cumulative AM and PM peak hourly traffic generation is summarised in **Table 4**.

**TABLE 4: ESTIMATED CUMULATIVE TRAFFIC GENERATION** 

Use	Туре	Scale	Peak	Generation Rate	Trips
Long day care (1)	Child Care	101	AM	0.8 per child	81 (41 in, 40 out)
Long-day care (1)	Centre	Children	PM	0.7 per child	71 (35 in, 36 out)
	Two	10 unito	AM	O.E. por dualling	10 (2 in, 8 out)
Residential <sup>(2)</sup>	Bedroom	19 units	PM	0.5 per dwelling	10 (8 in, 2 out)
	Three Bedroom	5 units	AM	0.65 per dwelling	3 (1 in, 2 out)
			PM		3 (2 in, 1 out)
Multi Dwelling Residential on	Three	07	AM	O CE non dualling	18 (4 in, 14 out)
Adjacent Properties (2)(3)	Bedroom	27 units	PM	0.65 per dwelling	18 (14 in, 4 out)
Total			АМ		112 (48 in, 64 out)
Total	-	-	PM		102 (59 in, 43 out)

### Notes:

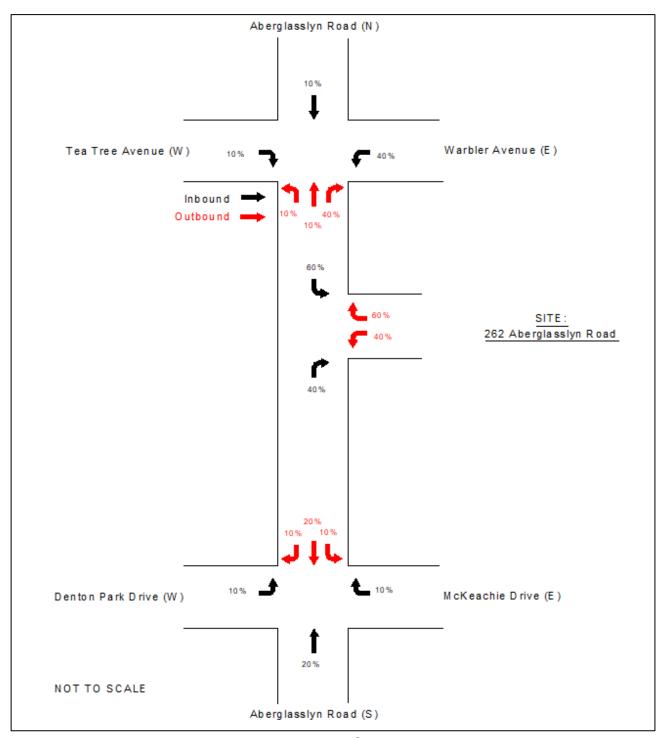
- (1) Assumes 50/50 inbound/outbound split.
- (2) Assumes 20% inbound and 80% outbound during AM peak: Vice versa for PM.
- (3) Assumes all units are three-bedroom as a conservative approach.

As shown, the expected traffic generation associated with the surrounding developments is in the order of **112** vehicle trips in the AM peak period (48 in, 64 out) and **102** vehicle trips in the PM peak period (59 in, 43 out).



## 4.3 Traffic Assignment

The road network, traffic surveys and locations of residential areas surrounding the site have been assessed and the following traffic assignment has been assumed for all traffic to and from the site:



**FIGURE 4: TRIP DISTRIBUTION** 



## 4.4 Traffic Impact

The traffic generation outlined in **Section 4.2 & 4.3** above has been added to the existing traffic volumes recorded. SIDRA INTERSECTION 9.1 was used to assess the intersections performance. The purpose of this assessment is to compare the existing intersection operations to the future scenario under the increased traffic load. The results of this assessment are shown in **Table 5**.

As shown, the intersection of Aberglasslyn Road / Tea Tree Avenue / Warbler Avenue, Aberglasslyn Road / McKeachie Drive / Denton Park Drive and Aberglasslyn Road / Site Driveway all retain the same overall level of service under future conditions with minimal delays and additional capacity, indicating that there will be negligible impact on the existing road network as a result of the proposed development.



# **TABLE 5: INTERSECTION PERFORMANCE (SIDRA INTERSECTION 9.1)**

Intersection	Peak Hour	Degree of Saturation <sup>(1)</sup>	Average Delay <sup>(2)</sup> (sec/veh)	Level of Service <sup>(3)(4)</sup>	Control Type	Worst Movement
			EXISTING PERFOR	RMANCE		
Tea Tree Avenue	AM	0.07	5.7 (Worst: 12.1)	A (Worst: B)	Roundabout	UT from Aberglasslyn Road
/ Warbler Avenue	PM	0.11	5.7 (Worst: 10.7)	<b>A</b> (Worst: B)	Roundabout	UT from Aberglasslyn Road
McKeachie Drive /	AM	0.38	6.2 (Worst: 11.7)	A (Worst: B)	Davidah aut	UT from Aberglasslyn Road
Denton Park Drive	PM	0.45	6.9 (Worst: 12.1)	A (Worst: B)	Roundabout	UT from Aberglasslyn Road
Aberglasslyn	AM	0.05	0.1 (Worst: 6)	NA (Worst: A)		RT from Site Driveway
Road / Site Driveway	PM	0.08	0.1 (Worst: 6.2)	NA (Worst: A)	Give Way	RT from Site Driveway
		FUTUR	E (POST DEVELOPMEN	IT) PERFORMANC	E	
Tea Tree Avenue	АМ	0.10	5.9 (Worst: 12.4)	A (Worst: B)	5	UT from Aberglasslyn Road
/ Warbler Avenue	PM	0.13	5.9 (Worst: 10.8)	A (Worst: B)	Roundabout	UT from Aberglasslyn Road
McKeachie Drive /	АМ	0.40	6.4 (Worst: 11.8)	A (Worst: B)		UT from Aberglasslyn Road
Denton Park Drive	PM	0.47	7.1 (Worst: 12.2)	A (Worst: B)	Roundabout	UT from Aberglasslyn Road
Aberglasslyn	АМ	0.05	0.4 (Worst: 6)	NA (Worst: A)	Oissa Massa	RT from Site Driveway
Road / Site Driveway	PM	0.09	0.3 (Worst: 6.2)	NA (Worst: A)	Give Way	RT from Site Driveway
		FUTUI	RE (10-YEAR GROWTH	) PERFORMANCE		
Tea Tree Avenue	AM	0.12	6 (Worst: 12.6)	<b>A</b> (Worst: B)	Poundahout	UT from Aberglasslyn Road
/ Warbler Avenue	PM	0.15	6 (Worst: 10.9)	<b>A</b> (Worst: B)	Roundabout	UT from Aberglasslyn Road
McKeachie Drive /	AM	0.49	6.9 (Worst: 12.2)	A (Worst: B)	Roundabout	UT from Aberglasslyn Road
Denton Park Drive	PM	0.59	7.8 (Worst: 13)	A (Worst: B)	Noundapout	UT from Aberglasslyn Road
Aberglasslyn	AM	0.06	0.5 (Worst: 6.1)	NA (Worst: A)	China Marin	RT from Site Driveway
Road / Site Driveway	PM	0.10	0.3 (Worst: 6.4)	NA (Worst: A)	Give Way	RT from Site Driveway

NOTES: Refer to Table 1.



## 4.5 Aberglasslyn Road / Site Driveway Turn Warrant

Reference is made to *Austroads Guide to Traffic Management Part 6: Intersections, Interchanges and Crossings*, which outlines intersection turn warrants. Turn treatments can consist of basic left and right turn treatment, short or full length left / right turn deceleration lanes. Consideration is given to the road design speed and traffic volumes.

The underlying basis for turn warrants is based upon the cost benefit ratio whereby the cost of providing the infrastructure upgrade is lower than that cost incurred due to crash costs over a particular design life.

Referring to Figure 2.25 of Austroads Guide to Traffic Management Part 6: Intersections, Interchanges and Crossings as shown in **Figure 5** below. The applicable left and right turn treatments for Aberglasslyn Road into the site driveway are summarised in **Table 6**.

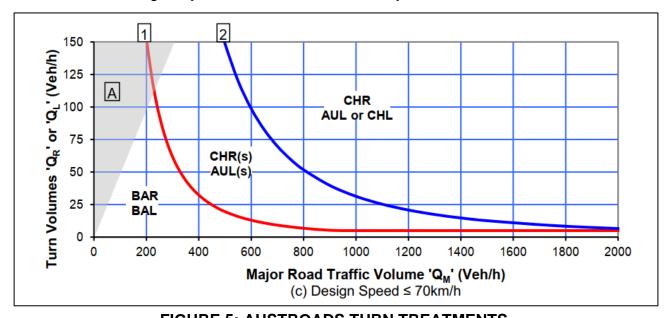
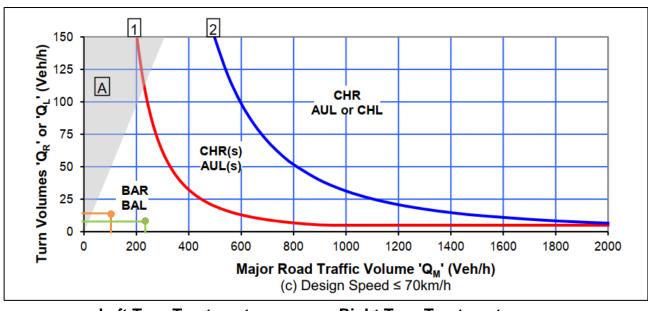


FIGURE 5: AUSTROADS TURN TREATMENTS
TABLE 6: AUSTROAD TURN WARRANT ASSESSMENT

Scenario	Peak Period	Turn	Turn Volume	Q <sub>M</sub> Value	Turn Warrant Treatment
	AM	Left Turn	2	87	BAL
Future Conditions	PM	(Q <sub>L</sub> )	6	78	BAL
Tuture Containons	AM	Right	1	154	BAR
	PM	Turn (Q <sub>R</sub> )	4	211	BAR

The worst-case scenario occurs during the PM peak period. The resultant warrant assessment during the PM peak period is depicted in **Figure 6**.





Left Turn Treatment Right Turn Treatment FIGURE 6: TREATMENT REQUIREMENT

As shown above, a BAR and BAL intersection turn treatment is warranted as part of the proposed development.

It should be noted that the assessment was conducted in line with requirements from Council within a letter dated 24 November 2022 and there is no required treatment lower than a BAR / BAL. It should also be noted that the warrant assessment and use of Austroads design guidelines strictly applies to intersections only, not driveways. The proposed driveway is provides access to 35 Class 1–2 car parking spaces from an arterial road. It is therefore a Category 2 access driveway in accordance with *AS2890.1:2004 Table 3.1*. Category 2 access driveways should not be treated as an intersection in accordance with Clause 3.1.1. As such, a BAR / BAL treatment is not strictly required.

Nonetheless, the upgrades to Aberglasslyn Road involve widening the road shoulder such that the additional road width would serve the same purpose as a BAR / BAL treatment, resulting in informal compliance with the Austroads Warrant Assessment.



## 5 CONCLUSION

In view of the foregoing, the subject residential townhouse proposal at 262 Aberglasslyn Road, Aberglasslyn (as depicted in **Annexure A**) is fully supportable in terms of its traffic and parking impacts. The following outcomes of this traffic impact assessment are relevant to note:

- The proposal includes the provision of **35** car parking spaces, comprised of **29** for residential use and six (**6**) for visitor use, satisfying the relevant controls applicable to the development, including Council's DCP requirements.
- Council's DCP does not require the provision of bicycle and motorcycle parking facilities.
- The parking areas of the site have been assessed against the relevant sections of AS2890.1:2004 and AS4299:1995 and have been found to satisfy the objectives of each standard. Swept path testing has been undertaken and is reproduced within Annexure D.
- The traffic generation of the proposed development has been estimated to be some 13 trips in the AM peak period (3 in, 10 out) and 13 trips in the PM peak period (10 in, 3 out). A cumulative traffic assessment has been undertaken and incorporates the traffic generated by the nearby proposed multi dwelling housing, the cumulative traffic generation has been estimated to be some 112 trips in the AM peak period (48 in, 64 out) and 102 trips in the PM peak period (59 in, 43 out). The impacts of the traffic generation have been modelled using SIDRA INTERSECTION 9.1, indicating that there will be no detrimental impact to the performance of the intersections as a result of the generated traffic.



ANNEXURE A: PROPOSED PLANS (1 SHEET)





ANNEXURE B: TRAFFIC SURVEY DATA (2 SHEETS)



GPS	-32.695796, 151.5350
Date:	Thu 03/11/22
Weather:	Fine
Suburban:	Aberglasslyn
Customer:	McLaren

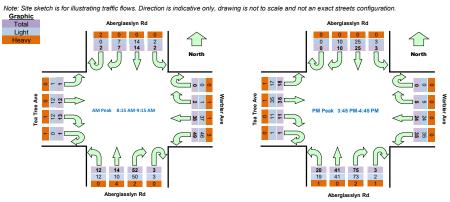
North:	Aberglasslyn Rd
East:	Warbler Ave
South:	Aberglasslyn Rd
West:	Tea Tree Ave

Survey	AM:	7:00 AM-9:30 AM
Period	PM:	2:30 PM-6:00 PM
Traffic	AM:	8:15 AM-9:15 AM
Peak	PM:	3:45 PM-4:45 PM

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	Period End	3	<b>R</b>	<b>SB</b>		<b>U</b>	<b>R</b>	WB	12	0	R	<b>NB</b>	2 2	0	R 3	<b>EB</b> 3	3	Hour 171	Peak
7:00	7:15		-		0	-	-	2		_	5			-		_			
7:15	7:30	0	5	9	0	0	0	4	3	0	2	2	0	0	5	1	0	167	
7:30	7:45	0	3	5	0	0	1	6	15	0	6	3	4	0	5	2	1	193	
7:45	8:00	0	2	8	0	0	0	7	12	1	1	9	0	0	0	0	0	201	
8:00	8:15	1	1	7	0	0	0	9	10	0	4	0	1	0	5	7	0	206	
8:15	8:30	0	2	3	1	0	2	14	14	0	10	3	4	1	1	2	0	222	Peak
8:30	8:45	0	3	5	0	0	0	13	16	0	8	5	1	0	5	3	0	201	
8:45	9:00	1	1	3	0	0	0	5	7	0	19	1	4	0	2	1	1		
9:00	9:15	1	1	3	1	0	0	6	12	3	15	5	3	0	5	6	0		
9:15	9:30	0	0	7	0	0	0	7	4	0	7	4	2	0	2	3	0		
14:30	14:45	2	2	7	0	0	0	3	7	1	8	3	2	0	3	3	3	222	
14:45	15:00	0	1	7	1	0	2	5	11	0	5	2	3	0	3	1	1	253	
15:00	15:15	0	1	12	1	0	2	3	11	2	6	3	3	0	6	6	5	280	
15:15	15:30	0	2	2	1	1	1	1	8	2	27	9	3	0	1	13	4	298	
15:30	15:45	0	0	9	0	0	0	5	5	2	25	10	6	0	2	6	5	300	
15:45	16:00	0	1	7	2	0	3	4	5	1	17	10	5	0	2	8	4	307	Peak
16:00	16:15	0	3	5	0	0	1	4	18	1	21	7	5	0	2	6	6	305	
16:15	16:30	0	3	3	0	0	1	11	3	1	18	12	5	1	3	12	4	289	
16:30	16:45	0	3	10	1	0	0	5	9	0	19	12	5	0	4	10	4	270	
16:45	17:00	0	1	7	2	0	1	4	8	0	16	12	4	0	2	5	5	253	
17:00	17:15	0	1	4	0	0	1	3	12	0	19	10	2	0	4	6	1	267	
17:15	17:30	0	3	6	1	1	1	4	8	0	9	9	1	0	4	7	4		
17:30	17:45	0	2	8	1	0	1	2	8	0	21	9	3	0	2	4	4		
17:45	18:00	0	2	7	0	0	1	5	12	0	20	14	9	0	2	6	3		

Peak	Time	North A	Approach	Aberglas	slyn Rd	East	Approac	h Warble	r Ave	South	Approach	Aberglass	lyn Rd	West	Approac	h Tea Tre	e Ave	Peak
Period Start	Period End	U	R	SB	L	U	R	WB	L	U	R	NB	L	U	R	EB	L	total
8:15	9:15	2	7	14	2	0	2	38	49	3	52	14	12	1	13	12	1	222
15:45	16:45	0	10	25	3	0	5	24	35	3	75	41	20	1	11	36	18	307





GPS -32.702386, 1

Date: Thu 03/11/22

Weather: Fine

Suburban: Aberglasslyn

Customer: McLaren

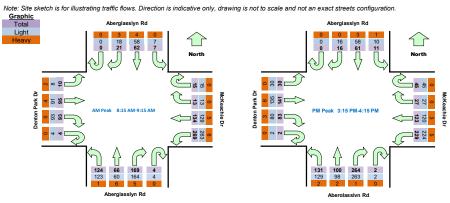
North:	Aberglasslyn Rd
East:	McKeachie Dr
South:	Aberglasslyn Rd
West:	Denton Park Dr

Survey	AM:	7:00 AM-9:30 AM
Period	PM:	2:30 PM-6:00 PM
Traffic	AM:	8:15 AM-9:15 AM
Peak	PM:	3:15 PM-4:15 PM

All Vehicles

All Verlieres																			
	me		Approach	Aberglas	slyn Rd	East	Approach		hie Dr	South	Approach	Aberglass	lyn Rd	West	Approach	Denton F	ark Dr		y Total
Period Start	Period End	U	R	SB	L	U	R	WB	L	U	R	NB	L	U	R	EB	L	Hour	Peak
7:00	7:15	0	2	22	3	5	1	17	39	0	17	4	8	0	18	7	2	697	1
7:15	7:30	0	4	13	2	2	0	19	56	0	20	2	8	0	22	13	2	763	
7:30	7:45	0	5	22	1	2	2	23	60	0	22	15	2	0	18	6	1	844	
7:45	8:00	0	7	18	0	4	3	21	61	0	19	8	14	0	30	22	3	1000	
8:00	8:15	0	3	22	1	3	1	31	64	1	16	5	16	0	30	18	0	1047	
8:15	8:30	0	5	17	2	4	2	31	76	1	31	13	21	1	26	13	1	1086	Peak
8:30	8:45	0	7	18	0	6	1	47	86	2	46	13	42	2	32	32	1	1029	
8:45	9:00	0	5	8	5	2	5	27	57	1	42	22	33	0	19	26	5		
9:00	9:15	0	4	19	0	3	5	29	50	0	50	18	28	1	16	24	3		
9:15	9:30	0	1	11	2	11	2	21	58	3	28	10	3	0	15	18	4		
14:30	14:45	0	3	16	1	7	3	22	53	0	44	9	14	1	29	16	6	1036	
14:45	15:00	0	4	15	1	7	2	34	55	0	38	6	14	0	26	27	3	1139	
15:00	15:15	0	7	30	1	8	7	24	34	0	56	12	20	0	23	24	2	1209	
15:15	15:30	0	4	17	1	12	9	28	49	0	71	24	43	0	21	43	10	1265	Peak
15:30	15:45	0	5	14	3	8	6	28	55	0	85	28	37	1	17	32	8	1236	
15:45	16:00	0	4	12	3	15	7	33	62	1	54	21	26	0	27	33	4	1230	
16:00	16:15	0	3	18	4	10	5	34	63	1	54	27	25	1	18	33	8	1232	
16:15	16:30	0	3	6	5	15	4	39	43	1	69	26	23	2	22	39	6	1235	
16:30	16:45	0	3	13	3	16	5	37	59	1	60	29	28	0	27	34	6	1208	
16:45	17:00	0	9	14	4	9	6	41	64	1	52	16	23	0	18	40	7	1176	
17:00	17:15	0	7	15	0	12	3	30	54	0	58	20	28	2	34	32	12	1164	
17:15	17:30	0	3	15	1	19	7	48	46	0	60	12	16	1	15	32	1		
17:30	17:45	0	6	13	0	11	2	43	46	0	54	27	24	1	19	34	9		
17:45	18:00	0	4	18	3	13	10	26	44	1	69	28	19	0	14	39	4		

Peak	Time	North A	Approach	Aberglas	slyn Rd	East	Approach	n McKeac	hie Dr	South	Approach	Aberglass	lyn Rd	West	Approach	Denton F	ark Dr	Peak
<b>Period Start</b>	Period End	U	R	SB	L	U	R	WB	L	U	R	NB	L	U	R	EB	L	total
8:15	9:15	0	21	62	7	15	13	134	269	4	169	66	124	4	93	95	10	1086
15:15	16:15	0	16	61	11	45	27	123	229	2	264	100	131	2	83	141	30	1265





ANNEXURE C: SIDRA RESULTS (18 SHEETS)

**♥** Site: 01 [EX AM Aberglasslyn Rd / Tea Tree Ave (Site Folder:

Existing )]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Tea Tree Avenue Existing Conditons AM Peak Period Site Category: (None) Roundabout

	Turn														
ID	rum	Mov Class		ows HV]	Flo Total F	rival ows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service		Back Of leue Dist] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South:	Aber	glasslyn F													
1	L2	All MCs	13	0.0	13	0.0	0.069	4.6	LOSA	0.3	2.5	0.18	0.58	0.18	48.1
2	T1	All MCs	15 2	28.6	15 2	28.6	0.069	5.2	LOSA	0.3	2.5	0.18	0.58	0.18	51.2
3	R2	All MCs	55	3.8	55	3.8	0.069	8.5	LOSA	0.3	2.5	0.18	0.58	0.18	47.7
3u	U	All MCs	3	0.0	3	0.0	0.069	10.3	LOS B	0.3	2.5	0.18	0.58	0.18	51.2
Approa	ach		85	7.4	85	7.4	0.069	7.5	LOSA	0.3	2.5	0.18	0.58	0.18	48.5
East: V	Narble	er Avenue	e (E)												
4	L2	All MCs	52	6.1	52	6.1	0.073	3.6	LOSA	0.4	2.6	0.16	0.42	0.16	49.5
5	T1	All MCs	40	2.6	40	2.6	0.073	3.7	LOSA	0.4	2.6	0.16	0.42	0.16	46.7
6	R2	All MCs	3 3	33.3	3 3	3.3	0.073	7.6	LOSA	0.4	2.6	0.16	0.42	0.16	48.2
Approa	ach		95	5.6	95	5.6	0.073	3.8	LOSA	0.4	2.6	0.16	0.42	0.16	48.2
North:	Abero	glasslyn F	Road (N	)											
7	L2	All MCs	2	0.0	2	0.0	0.022	4.7	LOSA	0.1	8.0	0.23	0.52	0.23	48.5
8	T1	All MCs	15	0.0	15	0.0	0.022	5.0	LOSA	0.1	0.8	0.23	0.52	0.23	52.5
9	R2	All MCs	7	0.0	7	0.0	0.022	8.6	LOSA	0.1	0.8	0.23	0.52	0.23	48.2
9u	U	All MCs	2 ^	100.	21	00.	0.022	12.1	LOS B	0.1	0.8	0.23	0.52	0.23	48.0
Approa	ach		26	0.8	26	0 8.0	0.022	6.6	LOSA	0.1	0.8	0.23	0.52	0.23	50.5
				0.0		0.0	0.022	0.0	2007.	0.1	0.0	0.20	0.02	0.20	00.0
		ree Avenu	, ,	0.0	4	0.0	0.004	2.0	1.00.4	0.4	0.0	0.00	0.54	0.00	40.4
10		All MCs		0.0		0.0	0.024	3.8	LOSA	0.1	0.8	0.22	0.51	0.22	48.4
11		All MCs		0.0	13		0.024	3.8	LOSA	0.1	0.8	0.22	0.51	0.22	45.6
12		All MCs		7.7 100.	14	00.	0.024	7.5	LOSA	0.1	0.8	0.22	0.51	0.22	47.8
12u	U	All MCs	I	0	Ι'	0	0.024	10.4	LOS B	0.1	8.0	0.22	0.51	0.22	43.7
Approa	ach		28	7.4	28	7.4	0.024	5.8	LOSA	0.1	0.8	0.22	0.51	0.22	46.6
All Veh	nicles		235	6.7	235	6.7	0.073	5.7	LOSA	0.4	2.6	0.18	0.50	0.18	48.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Project: \mte\_nas1\mte storage\Jobs\2022\221052\MTE SIDRA\22 11 11.sip9

**♥** Site: 01 [EX PM Aberglasslyn Rd / Tea Tree Ave (Site Folder:

Existing)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Tea Tree Avenue **Existing Conditions** PM Peak Period Site Category: (None) Roundabout

Vehic	cle Mo	ovement	Perfo	rma	nce										
Mov ID	Turn	Mov Class	[ Total	lows HV]		rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service		ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn F			VO1.//11		1,0			7011					1(11)/11
1	L2	All MCs	21	5.0	21	5.0	0.108	4.6	LOSA	0.5	3.9	0.16	0.57	0.16	48.4
2	T1	All MCs	43	0.0	43	0.0	0.108	4.8	LOSA	0.5	3.9	0.16	0.57	0.16	52.4
3	R2	All MCs	79	2.7	79	2.7	0.108	8.5	LOSA	0.5	3.9	0.16	0.57	0.16	48.1
3u	U	All MCs	3	33.3	3	33.3	0.108	10.7	LOS B	0.5	3.9	0.16	0.57	0.16	50.4
Appro	ach		146	2.9	146	2.9	0.108	6.9	LOSA	0.5	3.9	0.16	0.57	0.16	49.4
East:	Warbl	er Avenue	e (E)												
4	L2	All MCs	37	0.0	37	0.0	0.053	3.6	LOSA	0.3	1.8	0.18	0.44	0.18	49.6
5	T1	All MCs	25	0.0	25	0.0	0.053	3.7	LOSA	0.3	1.8	0.18	0.44	0.18	46.5
6	R2	All MCs	5	0.0	5	0.0	0.053	7.3	LOSA	0.3	1.8	0.18	0.44	0.18	49.2
Appro	ach		67	0.0	67	0.0	0.053	3.9	LOSA	0.3	1.8	0.18	0.44	0.18	48.4
North:	Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	3	0.0	3	0.0	0.035	5.0	LOSA	0.2	1.1	0.28	0.52	0.28	48.7
8	T1	All MCs	26	0.0	26	0.0	0.035	5.2	LOSA	0.2	1.1	0.28	0.52	0.28	52.6
9	R2	All MCs	11	0.0	11	0.0	0.035	8.8	LOSA	0.2	1.1	0.28	0.52	0.28	48.4
9u	U	All MCs	1	0.0	1	0.0	0.035	10.6	LOS B	0.2	1.1	0.28	0.52	0.28	51.9
Appro	ach		41	0.0	41	0.0	0.035	6.3	LOSA	0.2	1.1	0.28	0.52	0.28	51.1
West:	Tea T	ree Avenu	ue (W)												
10	L2	All MCs	20	5.3	20	5.3	0.060	4.1	LOSA	0.3	2.1	0.29	0.48	0.29	48.8
11	T1	All MCs	38	2.8	38	2.8	0.060	4.1	LOSA	0.3	2.1	0.29	0.48	0.29	46.0
12	R2	All MCs	12	0.0	12	0.0	0.060	7.6	LOSA	0.3	2.1	0.29	0.48	0.29	48.6
12u	U	All MCs	1	0.0	1	0.0	0.060	9.3	LOSA	0.3	2.1	0.29	0.48	0.29	45.5
Appro	ach		71	3.0	71	3.0	0.060	4.8	LOSA	0.3	2.1	0.29	0.48	0.29	47.2
All Ve	hicles		325	1.9	325	1.9	0.108	5.7	LOSA	0.5	3.9	0.21	0.51	0.21	48.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: 02 [EX AM Aberglasslyn Rd / Denton Park Dr (Site

Folder: Existing )]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Denton Park Drive **Existing Conditions** AM Peak Period Site Category: (None) Roundabout

Vehic	cle Mo	ovement	Perfo	rma	nce										
Mov ID	Turn	Mov Class	[ Total	lows HV]	Fl [ Total ]		Deg. Satn	Delay	Level of Service	Qu [ Veh.	Back Of eue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	: Aber	glasslyn f	veh/h Road (S		veh/h	%	v/c	sec		veh	m	_	_		km/h
1		All MCs	131	,	131	0.8	0.330	5.3	LOSA	2.2	15.5	0.46	0.58	0.46	48.1
2	T1	All MCs	69	9.1	69	9.1	0.330	5.7	LOSA	2.2	15.5	0.46	0.58	0.46	51.8
3	R2	All MCs	178	3.0	178	3.0	0.330	9.8	LOSA	2.2	15.5	0.46	0.58	0.46	47.7
3u	U	All MCs	4	0.0	4	0.0	0.330	11.7	LOS B	2.2	15.5	0.46	0.58	0.46	51.2
Appro	ach		382	3.3	382	3.3	0.330	7.5	LOSA	2.2	15.5	0.46	0.58	0.46	48.6
East:	McKe	achie Driv	re (E)												
4	L2	All MCs	283	2.2	283	2.2	0.382	4.3	LOSA	2.6	18.2	0.47	0.49	0.47	49.1
5	T1	All MCs	141	3.7	141	3.7	0.382	4.3	LOSA	2.6	18.2	0.47	0.49	0.47	46.1
6	R2	All MCs	14	0.0	14	0.0	0.382	8.4	LOSA	2.6	18.2	0.47	0.49	0.47	48.7
6u	U	All MCs	16	0.0	16	0.0	0.382	10.2	LOS B	2.6	18.2	0.47	0.49	0.47	45.6
Appro	ach		454	2.6	454	2.6	0.382	4.7	LOSA	2.6	18.2	0.47	0.49	0.47	48.0
North:	Aber	glasslyn F	Road (N	)											
7	L2	All MCs	7	0.0	7	0.0	0.101	6.1	LOSA	0.5	4.0	0.53	0.60	0.53	48.2
8	T1	All MCs	65	6.5	65	6.5	0.101	6.5	LOSA	0.5	4.0	0.53	0.60	0.53	52.0
9	R2	All MCs	22	14.3	22	14.3	0.101	11.1	LOS B	0.5	4.0	0.53	0.60	0.53	47.7
Appro	ach		95	7.8	95	7.8	0.101	7.5	LOSA	0.5	4.0	0.53	0.60	0.53	50.6
West:	Dento	on Park D	rive (W	)											
10	L2	All MCs	11	20.0	11 :	20.0	0.202	5.1	LOSA	1.2	8.4	0.49	0.57	0.49	47.4
11	T1	All MCs	100	4.2	100	4.2	0.202	4.7	LOSA	1.2	8.4	0.49	0.57	0.49	45.1
12	R2	All MCs	98	0.0	98	0.0	0.202	8.7	LOSA	1.2	8.4	0.49	0.57	0.49	47.6
12u	U	All MCs	4	0.0	4	0.0	0.202	10.5	LOS B	1.2	8.4	0.49	0.57	0.49	44.6
Appro	ach		213	3.0	213	3.0	0.202	6.7	LOSA	1.2	8.4	0.49	0.57	0.49	46.3
All Ve	hicles		1143	3.3	1143	3.3	0.382	6.2	LOSA	2.6	18.2	0.48	0.54	0.48	48.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: 02 [EX PM Aberglasslyn Rd / Denton Park Dr (Site

Folder: Existing )]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Denton Park Drive **Existing Conditions** PM Peak Period Site Category: (None) Roundabout

Vehic	cle Mo	ovemen	t Perfo	rma	nce										
Mov ID	Turn	Mov Class	F [ Total		Fl [ Total	rival lows HV] %	Deg. Satn v/c	Delay	Level of Service	Qu [ Veh.	Back Of eue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed
South	ı: Aber	glasslyn			veh/h	70	V/C	sec	_	veh	m				km/h
1		All MCs	,	,	138	1.5	0.451	5.6	LOS A	3.3	23.4	0.55	0.60	0.55	47.8
2	T1	All MCs	105	2.0	105	2.0	0.451	5.9	LOSA	3.3	23.4	0.55	0.60	0.55	51.5
3	R2	All MCs			278	0.4	0.451	10.1	LOS B	3.3	23.4	0.55	0.60	0.55	47.4
3u	U	All MCs	2	0.0	2	0.0	0.451	12.1	LOS B	3.3	23.4	0.55	0.60	0.55	50.8
Appro	ach		523	1.0	523	1.0	0.451	8.1	LOSA	3.3	23.4	0.55	0.60	0.55	48.3
East:	McKe	achie Dri	ve (E)												
4		All MCs	` ,	2.2	241	2.2	0.368	4.1	LOSA	2.5	17.6	0.44	0.50	0.44	48.8
5	T1	All MCs	129	2.4	129	2.4	0.368	4.2	LOSA	2.5	17.6	0.44	0.50	0.44	45.9
6	R2	All MCs	28	0.0	28	0.0	0.368	8.3	LOSA	2.5	17.6	0.44	0.50	0.44	48.5
6u	U	All MCs	47	0.0	47	0.0	0.368	10.0	LOS B	2.5	17.6	0.44	0.50	0.44	45.4
Appro	ach		446	1.9	446	1.9	0.368	5.0	LOSA	2.5	17.6	0.44	0.50	0.44	47.5
North	: Aber	glasslyn f	Road (N	l)											
7	L2	All MCs	12	9.1	12	9.1	0.111	7.5	LOSA	0.6	4.5	0.62	0.65	0.62	47.8
8	T1	All MCs	64	4.9	64	4.9	0.111	7.6	LOSA	0.6	4.5	0.62	0.65	0.62	51.6
9	R2	All MCs	17	0.0	17	0.0	0.111	11.7	LOS B	0.6	4.5	0.62	0.65	0.62	47.5
Appro	ach		93	4.5	93	4.5	0.111	8.3	LOSA	0.6	4.5	0.62	0.65	0.62	50.3
West:	Dento	on Park D	rive (W	')											
10	L2	All MCs	32	0.0	32	0.0	0.300	5.8	LOSA	1.9	13.7	0.65	0.63	0.65	47.6
11	T1	All MCs	148	4.3	148	4.3	0.300	6.0	LOSA	1.9	13.7	0.65	0.63	0.65	44.8
12	R2	All MCs	87	3.6	87	3.6	0.300	10.2	LOS B	1.9	13.7	0.65	0.63	0.65	47.1
12u	U	All MCs	2	0.0	2	0.0	0.300	11.8	LOS B	1.9	13.7	0.65	0.63	0.65	44.3
Appro	ach		269	3.5	269	3.5	0.300	7.4	LOSA	1.9	13.7	0.65	0.63	0.65	45.9
All Ve	hicles		1332	2.1	1332	2.1	0.451	6.9	LOSA	3.3	23.4	0.54	0.58	0.54	47.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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## ∇ Site: 03 [EX AM Aberglasslyn Rd / Site Driveway (Site Folder:

Existing)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Site Driveway **Existing Conditions** AM Peak Period Site Category: (None) Give-Way (Two-Way)

Vehic	cle Mo	ovement	Perfo	rma	nce										
Mov ID	Turn	Mov Class		ows HV]		rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh	ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn I	Road (S	5)											
2	T1	All MCs	85	7.4	85	7.4	0.046	0.0	LOSA	0.0	0.0	0.01	0.01	0.01	59.9
3	R2	All MCs	1	0.0	1	0.0	0.046	5.5	LOSA	0.0	0.0	0.01	0.01	0.01	57.1
Appro	ach		86	7.3	86	7.3	0.046	0.1	NA	0.0	0.0	0.01	0.01	0.01	59.9
East:	Site D	riveway (	E)												
4	L2	All MCs	1	0.0	1	0.0	0.002	5.8	LOSA	0.0	0.0	0.19	0.54	0.19	52.4
6	R2	All MCs	1	0.0	1	0.0	0.002	6.0	LOSA	0.0	0.0	0.19	0.54	0.19	52.1
Appro	ach		2	0.0	2	0.0	0.002	5.9	LOSA	0.0	0.0	0.19	0.54	0.19	52.2
North	Aber	glasslyn F	Road (N	)											
7	L2	All MCs	1	0.0	1	0.0	0.043	5.6	LOS A	0.0	0.0	0.00	0.01	0.00	57.4
8	T1	All MCs	80	5.3	80	5.3	0.043	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
Appro	ach		81	5.2	81	5.2	0.043	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.9
All Ve	hicles		169	6.2	169	6.2	0.046	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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## ∇ Site: 03 [EX PM Aberglasslyn Rd / Site Driveway (Site Folder:

Existing)]

### Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Site Driveway **Existing Conditions** PM Peak Period Site Category: (None) Give-Way (Two-Way)

Vehic	le Mo	ovement	t Perfo	rma	nce										
Mov ID	Turn	Mov Class		lows HV]		rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Que [ Veh. veh	ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn I	Road (S	S)											
2	T1	All MCs	159	2.0	159	2.0	0.083	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	60.0
3	R2	All MCs	1	0.0	1	0.0	0.083	5.5	LOS A	0.0	0.0	0.00	0.00	0.00	57.1
Appro	ach		160	2.0	160	2.0	0.083	0.0	NA	0.0	0.0	0.00	0.00	0.00	59.9
East:	Site D	riveway (	(E)												
4	L2	All MCs	1	0.0	1	0.0	0.002	5.7	LOS A	0.0	0.0	0.19	0.53	0.19	52.4
6	R2	All MCs	1	0.0	1	0.0	0.002	6.2	LOSA	0.0	0.0	0.19	0.53	0.19	52.1
Appro	ach		2	0.0	2	0.0	0.002	6.0	LOSA	0.0	0.0	0.19	0.53	0.19	52.2
North:	Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	1	0.0	1	0.0	0.037	5.5	LOSA	0.0	0.0	0.00	0.01	0.00	57.4
8	T1	All MCs	69	3.0	69	3.0	0.037	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	59.9
Appro	ach		71	3.0	71	3.0	0.037	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.9
All Ve	hicles		233	2.3	233	2.3	0.083	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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▼ Site: 01 [FU AM Aberglasslyn Rd / Tea Tree Ave (Site Folder:

Future)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Tea Tree Avenue Future Conditons AM Peak Period Site Category: (None) Roundabout

Vehicle Movement Performance													
Mov ID	Turn	Mov Class	Demand Flows [ Total HV ] veh/h %		Deg. Satn v/c	Aver. Delay sec		95% Ba Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn l		7011/11 /0	<b>V/</b> O	300		VOI1	- '''				KITI/TT
1	L2	All MCs	19 0.0	19 0.0	0.097	4.6	LOSA	0.5	3.5	0.18	0.58	0.18	49.4
2	T1	All MCs	21 20.0	21 20.0	0.097	5.1	LOSA	0.5	3.5	0.18	0.58	0.18	51.4
3	R2	All MCs	82 2.6	82 2.6	0.097	8.5	LOSA	0.5	3.5	0.18	0.58	0.18	49.0
3u	U	All MCs	3 0.0	3 0.0	0.097	10.3	LOS B	0.5	3.5	0.18	0.58	0.18	51.2
Appro	ach		125 5.0	125 5.0	0.097	7.4	LOSA	0.5	3.5	0.18	0.58	0.18	49.5
East:	Warbl	er Avenue	e (E)										
4	L2	All MCs	72 4.4	72 4.4	0.089	4.0	LOSA	0.4	3.2	0.18	0.44	0.18	50.3
5	T1	All MCs	40 2.6	40 2.6	0.089	3.7	LOSA	0.4	3.2	0.18	0.44	0.18	47.3
6	R2	All MCs	3 33.3	3 33.3	0.089	7.7	LOSA	0.4	3.2	0.18	0.44	0.18	48.8
Appro	ach		115 4.6	115 4.6	0.089	4.0	LOSA	0.4	3.2	0.18	0.44	0.18	49.1
North	: Aber	glasslyn F	Road (N)										
7	L2	All MCs	2 0.0	2 0.0	0.027	4.9	LOSA	0.1	0.9	0.27	0.52	0.27	48.6
8	T1	All MCs	20 0.0	20 0.0	0.027	5.2	LOSA	0.1	0.9	0.27	0.52	0.27	52.5
9	R2	All MCs	7 0.0	7 0.0	0.027	8.8	LOSA	0.1	0.9	0.27	0.52	0.27	48.2
9u	U	All MCs	2 <sup>100.</sup> 0	2 <sup>100.</sup> 0	0.027	12.4	LOS B	0.1	0.9	0.27	0.52	0.27	48.0
Appro	ach		32 6.7	32 6.7	0.027	6.5	LOSA	0.1	0.9	0.27	0.52	0.27	50.9
West:	West: Tea Tree Avenue (W)												
10	L2	All MCs	1 0.0	1 0.0	0.029	3.9	LOSA	0.1	1.0	0.27	0.53	0.27	48.7
11	T1	All MCs	13 0.0	13 0.0	0.029	4.0	LOSA	0.1	1.0	0.27	0.53	0.27	45.8
12	R2	All MCs	19 5.6	19 5.6	0.029	8.0	LOSA	0.1	1.0	0.27	0.53	0.27	48.2
12u	U	All MCs	1 <sup>100.</sup> 0	1 <sup>100.</sup> 0	0.029	10.7	LOS B	0.1	1.0	0.27	0.53	0.27	43.9
Appro	ach		34 6.3	34 6.3	0.029	6.4	LOSA	0.1	1.0	0.27	0.53	0.27	47.1
All Ve	hicles		305 5.2	305 5.2	0.097	5.9	LOSA	0.5	3.5	0.20	0.52	0.20	49.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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## Site: 01 [FU PM Aberglasslyn Rd / Tea Tree Ave (Site Folder:

Future)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Tea Tree Avenue **Future Conditions** PM Peak Period Site Category: (None) Roundabout

Vehic	cle Mo	ovement	Perfo	rma	nce										
Mov ID	Turn	Mov Class	[ Total	lows HV]		rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service		ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn F													
1	L2	All MCs	25	4.2	25	4.2	0.126	4.6	LOSA	0.6	4.6	0.16	0.57	0.16	49.1
2	T1	All MCs	47	0.0	47	0.0	0.126	4.8	LOSA	0.6	4.6	0.16	0.57	0.16	52.4
3	R2	All MCs	97	2.2	97	2.2	0.126	8.4	LOSA	0.6	4.6	0.16	0.57	0.16	48.8
3u	U	All MCs	3	33.3	3	33.3	0.126	10.7	LOS B	0.6	4.6	0.16	0.57	0.16	50.3
Appro	ach		173	2.4	173	2.4	0.126	6.9	LOSA	0.6	4.6	0.16	0.57	0.16	49.8
East:	Warbl	er Avenue	e (E)												
4	L2	All MCs	62	0.0	62	0.0	0.073	4.1	LOSA	0.4	2.5	0.20	0.46	0.20	50.6
5	T1	All MCs	25	0.0	25	0.0	0.073	3.8	LOSA	0.4	2.5	0.20	0.46	0.20	47.5
6	R2	All MCs	5	0.0	5	0.0	0.073	7.3	LOSA	0.4	2.5	0.20	0.46	0.20	50.3
Appro	ach		93	0.0	93	0.0	0.073	4.2	LOSA	0.4	2.5	0.20	0.46	0.20	49.7
North:	Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	3	0.0	3	0.0	0.041	5.1	LOSA	0.2	1.3	0.31	0.53	0.31	48.7
8	T1	All MCs	33	0.0	33	0.0	0.041	5.4	LOSA	0.2	1.3	0.31	0.53	0.31	52.6
9	R2	All MCs	11	0.0	11	0.0	0.041	9.0	LOSA	0.2	1.3	0.31	0.53	0.31	48.3
9u	U	All MCs	1	0.0	1	0.0	0.041	10.8	LOS B	0.2	1.3	0.31	0.53	0.31	51.9
Appro	ach		47	0.0	47	0.0	0.041	6.3	LOSA	0.2	1.3	0.31	0.53	0.31	51.3
West:	Tea T	ree Avenu	ue (W)												
10	L2	All MCs	20	5.3	20	5.3	0.067	4.2	LOSA	0.3	2.3	0.32	0.50	0.32	48.9
11	T1	All MCs	38	2.8	38	2.8	0.067	4.2	LOSA	0.3	2.3	0.32	0.50	0.32	46.1
12	R2	All MCs	18	0.0	18	0.0	0.067	8.2	LOSA	0.3	2.3	0.32	0.50	0.32	48.7
12u	U	All MCs	1	0.0	1	0.0	0.067	9.4	LOSA	0.3	2.3	0.32	0.50	0.32	45.6
Appro	ach		77	2.7	77	2.7	0.067	5.2	LOSA	0.3	2.3	0.32	0.50	0.32	47.4
All Ve	hicles		389	1.6	389	1.6	0.126	5.9	LOSA	0.6	4.6	0.22	0.52	0.22	49.5

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Site: 02 [FU AM Aberglasslyn Rd / Denton Park Dr (Site

Folder: Future)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Denton Park Drive **Future Conditions** AM Peak Period Site Category: (None) Roundabout

Vehicle Movement Performance															
Mov ID	Turn	Mov Class	[ Total	lows HV]	Fl [ Total ]		Deg. Satn v/c	Delay	Level of Service		Back Of eue Dist ]	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
veh/h % veh/h % South: Aberglasslyn Road (S)							V/C	sec		ven	m				KIII/II
1	L2	All MCs	131	0.8	131	0.8	0.343	5.3	LOSA	2.3	16.3	0.48	0.58	0.48	48.1
2	T1	All MCs	80	7.9	80	7.9	0.343	5.8	LOSA	2.3	16.3	0.48	0.58	0.48	51.8
3	R2	All MCs	178	3.0	178	3.0	0.343	9.9	LOSA	2.3	16.3	0.48	0.58	0.48	47.7
3u	U	All MCs	4	0.0	4	0.0	0.343	11.8	LOS B	2.3	16.3	0.48	0.58	0.48	51.2
Appro	ach		393	3.2	393	3.2	0.343	7.6	LOSA	2.3	16.3	0.48	0.58	0.48	48.7
East:	East: McKeachie Drive (E)														
4	L2	All MCs	283	2.2	283	2.2	0.395	4.5	LOSA	2.7	19.1	0.50	0.51	0.50	49.0
5	T1	All MCs	141	3.7	141	3.7	0.395	4.5	LOSA	2.7	19.1	0.50	0.51	0.50	46.0
6	R2	All MCs	19	0.0	19	0.0	0.395	9.0	LOSA	2.7	19.1	0.50	0.51	0.50	48.6
6u	U	All MCs	16	0.0	16	0.0	0.395	10.4	LOS B	2.7	19.1	0.50	0.51	0.50	45.5
Appro	ach		459	2.5	459	2.5	0.395	4.9	LOSA	2.7	19.1	0.50	0.51	0.50	47.9
North	Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	14	0.0	14	0.0	0.128	6.1	LOSA	0.7	5.0	0.53	0.60	0.53	50.0
8	T1	All MCs	79	5.3	79	5.3	0.128	6.5	LOSA	0.7	5.0	0.53	0.60	0.53	52.0
9	R2	All MCs	28	11.1	28	11.1	0.128	11.0	LOS B	0.7	5.0	0.53	0.60	0.53	48.5
Appro	ach		121	6.1	121	6.1	0.128	7.5	LOSA	0.7	5.0	0.53	0.60	0.53	50.9
West:	Dento	on Park D	rive (W	<b>'</b> )											
10	L2	All MCs	16	13.3	16	13.3	0.210	5.4	LOSA	1.2	8.8	0.51	0.58	0.51	47.7
11	T1	All MCs	100	4.2	100	4.2	0.210	4.8	LOSA	1.2	8.8	0.51	0.58	0.51	45.2
12	R2	All MCs	98	0.0	98	0.0	0.210	8.8	LOSA	1.2	8.8	0.51	0.58	0.51	47.6
12u	U	All MCs	4	0.0	4	0.0	0.210	10.6	LOS B	1.2	8.8	0.51	0.58	0.51	44.7
Appro	ach		218	2.9	218	2.9	0.210	6.8	LOSA	1.2	8.8	0.51	0.58	0.51	46.4
All Ve	hicles		1191	3.2	1191	3.2	0.395	6.4	LOSA	2.7	19.1	0.50	0.55	0.50	48.1

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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▼ Site: 02 [FU PM Aberglasslyn Rd / Denton Park Dr (Site)

Folder: Future)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Denton Park Drive **Future Conditions** PM Peak Period Site Category: (None) Roundabout

Vehic	cle Mo	ovement	Perfo	rma	nce										
Mov ID	Turn	Mov Class		lows HV]		rival lows HV] %	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% B Qu [ Veh. veh	ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn F				,,	.,,								11111111
1	L2	All MCs	138	1.5	138	1.5	0.467	5.7	LOSA	3.5	24.6	0.57	0.60	0.57	47.7
2	T1	All MCs	118	1.8	118	1.8	0.467	6.0	LOSA	3.5	24.6	0.57	0.60	0.57	51.5
3	R2	All MCs	278	0.4	278	0.4	0.467	10.2	LOS B	3.5	24.6	0.57	0.60	0.57	47.4
3u	U	All MCs	2	0.0	2	0.0	0.467	12.2	LOS B	3.5	24.6	0.57	0.60	0.57	50.8
Appro	ach		536	1.0	536	1.0	0.467	8.1	LOSA	3.5	24.6	0.57	0.60	0.57	48.3
East:	McKe	achie Driv	re (E)												
4	L2	All MCs	241	2.2	241	2.2	0.378	4.2	LOSA	2.6	18.2	0.46	0.51	0.46	48.8
5	T1	All MCs	129	2.4	129	2.4	0.378	4.3	LOSA	2.6	18.2	0.46	0.51	0.46	45.9
6	R2	All MCs	35	0.0	35	0.0	0.378	8.7	LOSA	2.6	18.2	0.46	0.51	0.46	48.4
6u	U	All MCs	47	0.0	47	0.0	0.378	10.2	LOS B	2.6	18.2	0.46	0.51	0.46	45.3
Appro	ach		453	1.9	453	1.9	0.378	5.2	LOSA	2.6	18.2	0.46	0.51	0.46	47.5
North	: Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	16	6.7	16	6.7	0.132	7.5	LOSA	0.7	5.4	0.63	0.65	0.63	48.8
8	T1	All MCs	74	4.3	74	4.3	0.132	7.6	LOSA	0.7	5.4	0.63	0.65	0.63	51.5
9	R2	All MCs	21	0.0	21	0.0	0.132	11.7	LOS B	0.7	5.4	0.63	0.65	0.63	48.2
Appro	ach		111	3.8	111	3.8	0.132	8.4	LOSA	0.7	5.4	0.63	0.65	0.63	50.5
West:	Dento	on Park D	rive (W	)											
10	L2	All MCs	38	0.0	38	0.0	0.312	6.2	LOSA	2.0	14.4	0.67	0.64	0.67	47.7
11	T1	All MCs	148	4.3	148	4.3	0.312	6.2	LOSA	2.0	14.4	0.67	0.64	0.67	44.9
12	R2	All MCs	87	3.6	87	3.6	0.312	10.3	LOS B	2.0	14.4	0.67	0.64	0.67	47.2
12u	U	All MCs	2	0.0	2	0.0	0.312	11.9	LOS B	2.0	14.4	0.67	0.64	0.67	44.4
Appro	ach		276	3.4	276	3.4	0.312	7.5	LOSA	2.0	14.4	0.67	0.64	0.67	45.9
All Ve	hicles		1375	2.0	1375	2.0	0.467	7.1	LOSA	3.5	24.6	0.56	0.58	0.56	47.7

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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# ∇ Site: 03 [FU AM Aberglasslyn Rd / Site Driveway (Site Folder:

Future)]

#### Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Site Driveway **Future Conditions** AM Peak Period Site Category: (None) Give-Way (Two-Way)

Vehic	Vehicle Movement Performance  Mov Turn Mov Demand Arrival Deg. Aver. Level of 95% Back Of Prop. Eff. Aver. Aver.														
Mov ID	Turn	Mov Class	FI	lows HV]		lows	Deg. Satn v/c	Aver. Delay sec	Level of Service		ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn	Road (S	S)											
2	T1	All MCs	85	7.4	85	7.4	0.046	0.0	LOSA	0.0	0.0	0.01	0.01	0.01	59.9
3	R2	All MCs	1	0.0	1	0.0	0.046	5.5	LOS A	0.0	0.0	0.01	0.01	0.01	57.1
Appro	ach		86	7.3	86	7.3	0.046	0.1	NA	0.0	0.0	0.01	0.01	0.01	59.9
East:	Site D	riveway (	E)												
4	L2	All MCs	4	0.0	4	0.0	0.008	5.8	LOS A	0.0	0.2	0.20	0.55	0.20	52.4
6	R2	All MCs	6	0.0	6	0.0	0.008	6.0	LOS A	0.0	0.2	0.20	0.55	0.20	52.1
Appro	ach		11	0.0	11	0.0	0.008	5.9	LOSA	0.0	0.2	0.20	0.55	0.20	52.2
North:	Aber	glasslyn F	Road (N	)											
7	L2	All MCs	2	0.0	2	0.0	0.044	5.6	LOSA	0.0	0.0	0.00	0.02	0.00	57.3
8	T1	All MCs	80	5.3	80	5.3	0.044	0.0	LOS A	0.0	0.0	0.00	0.02	0.00	59.8
Appro	ach		82	5.1	82	5.1	0.044	0.2	NA	0.0	0.0	0.00	0.02	0.00	59.8
All Ve	hicles		179	5.9	179	5.9	0.046	0.4	NA	0.0	0.2	0.01	0.04	0.01	59.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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# ∇ Site: 03 [FU PM Aberglasslyn Rd / Site Driveway (Site Folder:

Future)]

#### Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Site Driveway **Future Conditions** PM Peak Period Site Category: (None) Give-Way (Two-Way)

Vehic	Vehicle Movement Performance  Mov Turn Mov Demand Arrival Deg. Aver. Level of 95% Back Of Prop. Eff. Aver. Aver.														
Mov ID	Turn	Mov Class	FI	lows HV]		ows	Deg. Satn v/c	Aver. Delay sec	Level of Service		Back Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn	Road (S	S)											
2	T1	All MCs	159	2.0	159	2.0	0.085	0.0	LOSA	0.0	0.2	0.01	0.02	0.01	59.8
3	R2	All MCs	4	0.0	4	0.0	0.085	5.5	LOS A	0.0	0.2	0.01	0.02	0.01	57.0
Appro	ach		163	1.9	163	1.9	0.085	0.1	NA	0.0	0.2	0.01	0.02	0.01	59.7
East:	Site D	riveway (	(E)												
4	L2	All MCs	1	0.0	1	0.0	0.003	5.7	LOSA	0.0	0.1	0.21	0.54	0.21	52.3
6	R2	All MCs	2	0.0	2	0.0	0.003	6.2	LOSA	0.0	0.1	0.21	0.54	0.21	52.0
Appro	ach		3	0.0	3	0.0	0.003	6.1	LOS A	0.0	0.1	0.21	0.54	0.21	52.1
North:	Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	6	0.0	6	0.0	0.040	5.5	LOSA	0.0	0.0	0.00	0.05	0.00	57.1
8	T1	All MCs	69	3.0	69	3.0	0.040	0.0	LOSA	0.0	0.0	0.00	0.05	0.00	59.5
Appro	ach		76	2.8	76	2.8	0.040	0.5	NA	0.0	0.0	0.00	0.05	0.00	59.3
All Ve	hicles		242	2.2	242	2.2	0.085	0.3	NA	0.0	0.2	0.01	0.03	0.01	59.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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▼ Site: 01 [FU AM Aberglasslyn Rd / Tea Tree Ave - 10 year

growth (Site Folder: Future - 10 year growth)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Tea Tree Avenue Future (10 year growth) Conditons AM Peak Period Site Category: (None) Roundabout

Design Life Analysis (Final Year): Results for 10 years

Vehicle Movement PerformanceMov Turn MovDemandArrivalDeg. Aver. Level of95% Back OfProp.Eff. Aver. Aver.													
ID		Class	Flows [ Total HV ] veh/h %	Flows [ Total HV ]	Deg. Satn v/c	Aver. Delay sec	Level of Service	95% Ba Que [ Veh. veh		Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	ı: Aber	glasslyn l	Road (S)										
1	L2	All MCs	23 0.0	23 0.0	0.118	4.7	LOSA	0.6	4.4	0.21	0.58	0.21	49.3
2	T1	All MCs	25 20.0	25 20.0	0.118	5.2	LOSA	0.6	4.4	0.21	0.58	0.21	51.4
3	R2	All MCs	99 2.6	99 2.6	0.118	8.6	LOSA	0.6	4.4	0.21	0.58	0.21	49.0
3u	U	All MCs	4 0.0	4 0.0	0.118	10.3	LOS B	0.6	4.4	0.21	0.58	0.21	51.2
Appro	oach		150 5.0	150 5.0	0.118	7.5	LOSA	0.6	4.4	0.21	0.58	0.21	49.5
East:	Warbl	er Avenue	∋ (E)										
4	L2	All MCs	86 4.4	86 4.4	0.108	4.0	LOSA	0.5	4.0	0.21	0.44	0.21	50.2
5	T1	All MCs	48 2.6	48 2.6	0.108	3.8	LOSA	0.5	4.0	0.21	0.44	0.21	47.2
6	R2	All MCs	4 33.3	4 33.3	0.108	7.7	LOSA	0.5	4.0	0.21	0.44	0.21	48.7
Appro	oach		138 4.6	138 4.6	0.108	4.0	LOSA	0.5	4.0	0.21	0.44	0.21	49.1
North	: Aber	glasslyn F	Road (N)										
7	L2	All MCs	3 0.0	3 0.0	0.033	5.0	LOSA	0.2	1.2	0.30	0.53	0.30	48.5
8	T1	All MCs	24 0.0	24 0.0	0.033	5.3	LOSA	0.2	1.2	0.30	0.53	0.30	52.4
9	R2	All MCs	9 0.0	9 0.0	0.033	8.9	LOSA	0.2	1.2	0.30	0.53	0.30	48.2
9u	U	All MCs	3 100. 0	3 100. 0	0.033	12.6	LOS B	0.2	1.2	0.30	0.53	0.30	47.9
Appro	oach		38 6.7	38 6.7	0.033	6.6	LOSA	0.2	1.2	0.30	0.53	0.30	50.8
West	Tea T	ree Aven	ue (W)										
10	L2	All MCs	1 0.0	1 0.0	0.036	4.0	LOSA	0.2	1.3	0.30	0.54	0.30	48.6
11	T1	All MCs	15 0.0	15 0.0	0.036	4.1	LOSA	0.2	1.3	0.30	0.54	0.30	45.8
12	R2	All MCs	23 5.6	23 5.6	0.036	8.1	LOSA	0.2	1.3	0.30	0.54	0.30	48.1
12u	U	All MCs	1 <sup>100.</sup> 0	1 <sup>100.</sup> 0	0.036	10.9	LOS B	0.2	1.3	0.30	0.54	0.30	43.9
Appro	oach		40 6.3	40 6.3	0.036	6.6	LOSA	0.2	1.3	0.30	0.54	0.30	47.1
All Ve	hicles		366 5.2	366 5.2	0.118	6.0	LOSA	0.6	4.4	0.23	0.52	0.23	49.2

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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▼ Site: 01 [FU PM Aberglasslyn Rd / Tea Tree Ave - 10 year

growth (Site Folder: Future - 10 year growth)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Tea Tree Avenue Future (10 year growth) Conditons PM Peak Period Site Category: (None) Roundabout

Design Life Analysis (Final Year): Results for 10 years

Vehicle Movement PerformanceMov Turn MovDemandArrivalDeg. Aver. Level of95% Back OfProp. Eff. Aver. Aver.															
Mov ID	Turn	Mov Class	F	lows HV]	Fl [ Total	lows	Deg. Satn v/c	Aver. Delay sec	Level of Service		ack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn l	Road (S	3)											
1	L2	All MCs	30	4.2	30	4.2	0.153	4.6	LOSA	0.8	5.8	0.18	0.57	0.18	49.0
2	T1	All MCs	57	0.0	57	0.0	0.153	4.9	LOSA	0.8	5.8	0.18	0.57	0.18	52.3
3	R2	All MCs	116	2.2	116	2.2	0.153	8.5	LOSA	0.8	5.8	0.18	0.57	0.18	48.8
3u	U	All MCs	4	33.3	4	33.3	0.153	10.7	LOS B	0.8	5.8	0.18	0.57	0.18	50.3
Appro	ach		207	2.4	207	2.4	0.153	7.0	LOSA	8.0	5.8	0.18	0.57	0.18	49.7
East:	Warbl	er Avenue	e (E)												
4	L2	All MCs	75	0.0	75	0.0	0.089	4.2	LOSA	0.4	3.1	0.23	0.46	0.23	50.6
5	T1	All MCs	30	0.0	30	0.0	0.089	3.8	LOSA	0.4	3.1	0.23	0.46	0.23	47.4
6	R2	All MCs	6	0.0	6	0.0	0.089	7.4	LOSA	0.4	3.1	0.23	0.46	0.23	50.2
Appro	ach		111	0.0	111	0.0	0.089	4.3	LOSA	0.4	3.1	0.23	0.46	0.23	49.7
North	: Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	4	0.0	4	0.0	0.050	5.3	LOSA	0.2	1.7	0.34	0.54	0.34	48.6
8	T1	All MCs	39	0.0	39	0.0	0.050	5.5	LOSA	0.2	1.7	0.34	0.54	0.34	52.5
9	R2	All MCs	13	0.0	13	0.0	0.050	9.1	LOSA	0.2	1.7	0.34	0.54	0.34	48.2
9u	U	All MCs	1	0.0	1	0.0	0.050	10.9	LOS B	0.2	1.7	0.34	0.54	0.34	51.8
Appro	ach		57	0.0	57	0.0	0.050	6.4	LOSA	0.2	1.7	0.34	0.54	0.34	51.2
West	Tea T	ree Aveni	ue (W)												
10	L2	All MCs	24	5.3	24	5.3	0.083	4.4	LOSA	0.4	2.9	0.35	0.51	0.35	48.8
11	T1	All MCs	45	2.8	45	2.8	0.083	4.4	LOSA	0.4	2.9	0.35	0.51	0.35	46.0
12	R2	All MCs	21	0.0	21	0.0	0.083	8.4	LOSA	0.4	2.9	0.35	0.51	0.35	48.6
12u	U	All MCs	1	0.0	1	0.0	0.083	9.5	LOSA	0.4	2.9	0.35	0.51	0.35	45.5
Appro	ach		92	2.7	92	2.7	0.083	5.4	LOSA	0.4	2.9	0.35	0.51	0.35	47.3
All Ve	hicles		467	1.6	467	1.6	0.153	6.0	LOSA	0.8	5.8	0.25	0.53	0.25	49.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

▼ Site: 02 [FU AM Aberglasslyn Rd / Denton Park Dr - 10 year

growth (Site Folder: Future - 10 year growth)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Denton Park Drive Future (10 year growth) Conditions AM Peak Period Site Category: (None) Roundabout

Design Life Analysis (Final Year): Results for 10 years

Vehicle Movement PerformanceMov Turn MovDemandArrivalDeg. Aver. Level of95% Back OfProp.Eff. Aver. Aver.															
Mov ID	Turn	Mov Class	Fl [ Total	lows HV]		ows	Deg. Satn v/c		Level of Service		Back Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn l	Road (S	S)											
1	L2	All MCs	157	8.0	157	8.0	0.430	5.8	LOSA	3.1	22.3	0.57	0.61	0.57	47.9
2	T1	All MCs	96	7.9	96	7.9	0.430	6.2	LOSA	3.1	22.3	0.57	0.61	0.57	51.5
3	R2	All MCs	213	3.0	213	3.0	0.430	10.4	LOS B	3.1	22.3	0.57	0.61	0.57	47.5
3u	U	All MCs	5	0.0	5	0.0	0.430	12.2	LOS B	3.1	22.3	0.57	0.61	0.57	50.9
Appro	ach		471	3.2	471	3.2	0.430	8.0	LOSA	3.1	22.3	0.57	0.61	0.57	48.4
East:	McKe	achie Driv	/e (E)												
4	L2	All MCs	340	2.2	340	2.2	0.493	5.0	LOSA	3.7	26.2	0.59	0.55	0.59	48.7
5	T1	All MCs	169	3.7	169	3.7	0.493	5.0	LOSA	3.7	26.2	0.59	0.55	0.59	45.8
6	R2	All MCs	23	0.0	23	0.0	0.493	9.5	LOSA	3.7	26.2	0.59	0.55	0.59	48.4
6u	U	All MCs	19	0.0	19	0.0	0.493	10.8	LOS B	3.7	26.2	0.59	0.55	0.59	45.2
Appro	ach		551	2.5	551	2.5	0.493	5.4	LOSA	3.7	26.2	0.59	0.55	0.59	47.6
North	: Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	16	0.0	16	0.0	0.165	6.7	LOSA	0.9	6.8	0.60	0.63	0.60	49.7
8	T1	All MCs	95	5.3	95	5.3	0.165	7.2	LOSA	0.9	6.8	0.60	0.63	0.60	51.6
9	R2	All MCs	34	11.1	34	11.1	0.165	11.6	LOS B	0.9	6.8	0.60	0.63	0.60	48.3
Appro	ach		145	6.1	145	6.1	0.165	8.2	LOSA	0.9	6.8	0.60	0.63	0.60	50.6
West:	Dento	on Park D	rive (W	)											
10	L2	All MCs	19	13.3	19	13.3	0.268	5.9	LOSA	1.7	11.9	0.58	0.61	0.58	47.4
11	T1	All MCs	120	4.2	120	4.2	0.268	5.3	LOSA	1.7	11.9	0.58	0.61	0.58	44.9
12	R2	All MCs	117	0.0	117	0.0	0.268	9.3	LOSA	1.7	11.9	0.58	0.61	0.58	47.4
12u	U	All MCs	5	0.0	5	0.0	0.268	11.1	LOS B	1.7	11.9	0.58	0.61	0.58	44.4
Appro	ach		261	2.9	261	2.9	0.268	7.2	LOSA	1.7	11.9	0.58	0.61	0.58	46.1
All Ve	hicles		1429	3.2	1429	3.2	0.493	6.9	LOSA	3.7	26.2	0.59	0.59	0.59	47.9

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

🦁 Site: 02 [FU PM Aberglasslyn Rd / Denton Park Dr - 10 year

growth (Site Folder: Future - 10 year growth)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Denton Park Drive Future (10 year growth) Conditions PM Peak Period Site Category: (None) Roundabout

Design Life Analysis (Final Year): Results for 10 years

Vehicle Movement Performance  Mov Turn Mov Demand Arrival Deg. Aver. Level of 95% Back Of Prop. Eff. Aver. Aver.															
Mov ID	Turn	Mov Class	F	lows HV]	Fl [ Total ]	ows	Deg. Satn v/c	Aver. Delay sec	Level of Service		Back Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn l	Road (S	S)											
1	L2	All MCs	165	1.5	165	1.5	0.587	6.5	LOSA	5.1	36.3	0.70	0.65	0.71	47.4
2	T1	All MCs	141	1.8	141	1.8	0.587	6.8	LOSA	5.1	36.3	0.70	0.65	0.71	51.1
3	R2	All MCs	333	0.4	333	0.4	0.587	11.0	LOS B	5.1	36.3	0.70	0.65	0.71	47.0
3u	U	All MCs	3	0.0	3	0.0	0.587	13.0	LOS B	5.1	36.3	0.70	0.65	0.71	50.4
Appro	ach		643	1.0	643	1.0	0.587	8.9	LOSA	5.1	36.3	0.70	0.65	0.71	48.0
East:	McKe	achie Driv	/e (E)												
4	L2	All MCs	289	2.2	289	2.2	0.470	4.7	LOSA	3.5	24.9	0.56	0.54	0.56	48.5
5	T1	All MCs	155	2.4	155	2.4	0.470	4.7	LOSA	3.5	24.9	0.56	0.54	0.56	45.6
6	R2	All MCs	42	0.0	42	0.0	0.470	9.1	LOSA	3.5	24.9	0.56	0.54	0.56	48.2
6u	U	All MCs	57	0.0	57	0.0	0.470	10.6	LOS B	3.5	24.9	0.56	0.54	0.56	45.1
Appro	ach		543	1.9	543	1.9	0.470	5.6	LOSA	3.5	24.9	0.56	0.54	0.56	47.3
North	: Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	19	6.7	19	6.7	0.177	8.5	LOSA	1.1	7.7	0.71	0.69	0.71	48.3
8	T1	All MCs	88	4.3	88	4.3	0.177	8.7	LOSA	1.1	7.7	0.71	0.69	0.71	51.0
9	R2	All MCs	25	0.0	25	0.0	0.177	12.7	LOS B	1.1	7.7	0.71	0.69	0.71	47.7
Appro	ach		133	3.8	133	3.8	0.177	9.4	LOSA	1.1	7.7	0.71	0.69	0.71	49.9
West	Dento	on Park D	rive (W	)											
10	L2	All MCs	45	0.0	45	0.0	0.417	7.2	LOSA	2.9	21.1	0.79	0.70	0.79	47.2
11	T1	All MCs	178	4.3	178	4.3	0.417	7.2	LOSA	2.9	21.1	0.79	0.70	0.79	44.4
12	R2	All MCs	105	3.6	105	3.6	0.417	11.3	LOS B	2.9	21.1	0.79	0.70	0.79	46.7
12u	U	All MCs	3	0.0	3	0.0	0.417	12.9	LOS B	2.9	21.1	0.79	0.70	0.79	43.9
Appro	pproach 331 3.4 331 3.4				3.4	0.417	8.5	LOSA	2.9	21.1	0.79	0.70	0.79	45.5	
All Ve	hicles		1650	2.0	1650	2.0	0.587	7.8	LOSA	5.1	36.3	0.67	0.63	0.67	47.4

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Roundabout LOS Method: SIDRA Roundabout LOS.

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

V Site: 03 [FU AM Aberglasslyn Rd / Site Driveway - 10 year

growth (Site Folder: Future - 10 year growth)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Site Driveway Future (10 year growth) Conditions AM Peak Period Site Category: (None) Give-Way (Two-Way)

Design Life Analysis (Final Year): Results for 10 years

Vehic	Vehicle Movement Performance  Mov Turn Mov Demand Arrival Deg. Aver. Level of 95% Back Of Prop. Eff. Aver. Aver.														
Mov ID	Turn	Mov Class	FI	lows HV]		ows	Deg. Satn v/c	Aver. Delay sec	Level of Service		lack Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn l	Road (S	3)											
2	T1	All MCs	102	7.4	102	7.4	0.056	0.0	LOSA	0.0	0.1	0.01	0.01	0.01	59.9
3	R2	All MCs	1	0.0	1	0.0	0.056	5.5	LOS A	0.0	0.1	0.01	0.01	0.01	57.1
Appro	ach		104	7.3	104	7.3	0.056	0.1	NA	0.0	0.1	0.01	0.01	0.01	59.9
East:	East: Site Driveway (E)														
4	L2	All MCs	5	0.0	5	0.0	0.010	5.8	LOSA	0.0	0.2	0.22	0.55	0.22	52.3
6	R2	All MCs	8	0.0	8	0.0	0.010	6.1	LOS A	0.0	0.2	0.22	0.55	0.22	52.0
Appro	ach		13	0.0	13	0.0	0.010	6.0	LOSA	0.0	0.2	0.22	0.55	0.22	52.1
North	Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	3	0.0	3	0.0	0.052	5.6	LOSA	0.0	0.0	0.00	0.02	0.00	57.3
8	T1	All MCs	96	5.3	96	5.3	0.052	0.0	LOS A	0.0	0.0	0.00	0.02	0.00	59.8
Appro	ach		99	5.1	99	5.1	0.052	0.2	NA	0.0	0.0	0.00	0.02	0.00	59.8
All Ve	hicles		215	5.9	215	5.9	0.056	0.5	NA	0.0	0.2	0.02	0.04	0.02	59.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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V Site: 03 [FU PM Aberglasslyn Rd / Site Driveway - 10 year

growth (Site Folder: Future - 10 year growth)]

Output produced by SIDRA INTERSECTION Version: 9.1.3.210

Aberglasslyn Road / Site Driveway Future (10 year growth) Conditions PM Peak Period Site Category: (None) Give-Way (Two-Way)

Design Life Analysis (Final Year): Results for 10 years

Vehic	<b>Vehicle Movement Performance</b> Mov Turn Mov Demand Arrival Deg. Aver. Level of 95% Back Of Prop. Eff. Aver. Aver.														
Mov ID	Turn	Mov Class	FI	lows HV]		ows	Deg. Satn v/c	Aver. Delay sec	Level of Service		Back Of eue Dist ] m	Prop. Que	Eff. Stop Rate	Aver. No. of Cycles	Aver. Speed km/h
South	: Aber	glasslyn I	Road (S	3)											
2	T1	All MCs	191	2.0	191	2.0	0.102	0.0	LOSA	0.0	0.2	0.01	0.02	0.01	59.8
3	R2	All MCs	5	0.0	5	0.0	0.102	5.6	LOS A	0.0	0.2	0.01	0.02	0.01	57.0
Appro	Approach			1.9	196	1.9	0.102	0.1	NA	0.0	0.2	0.01	0.02	0.01	59.7
East:	East: Site Driveway (E)														
4	L2	All MCs	1	0.0	1	0.0	0.003	5.8	LOSA	0.0	0.1	0.24	0.54	0.24	52.2
6	R2	All MCs	3	0.0	3	0.0	0.003	6.4	LOSA	0.0	0.1	0.24	0.54	0.24	52.0
Appro	ach		4	0.0	4	0.0	0.003	6.2	LOSA	0.0	0.1	0.24	0.54	0.24	52.1
North	: Aber	glasslyn F	Road (N	l)											
7	L2	All MCs	8	0.0	8	0.0	0.048	5.6	LOSA	0.0	0.0	0.00	0.05	0.00	57.0
8	T1	All MCs	83	3.0	83	3.0	0.048	0.0	LOS A	0.0	0.0	0.00	0.05	0.00	59.5
Appro	ach		91	2.8	91	2.8	0.048	0.5	NA	0.0	0.0	0.00	0.05	0.00	59.3
All Ve	hicles		291	2.2	291	2.2	0.102	0.3	NA	0.0	0.2	0.01	0.03	0.01	59.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Options tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

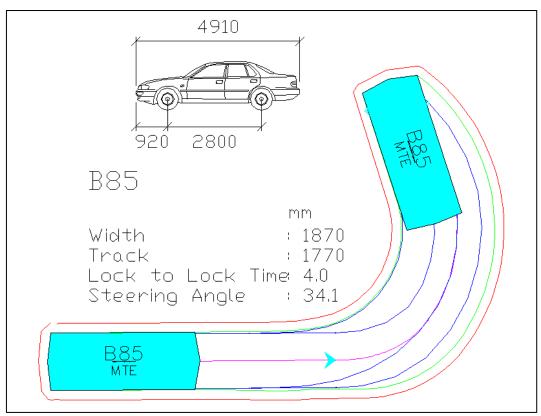
Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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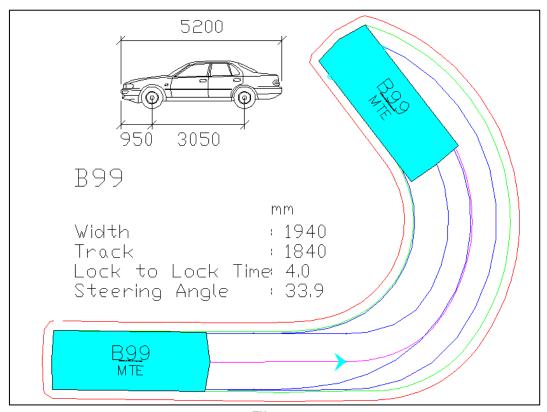
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ANNEXURE D: SWEPT PATH TESTING (6 SHEETS)

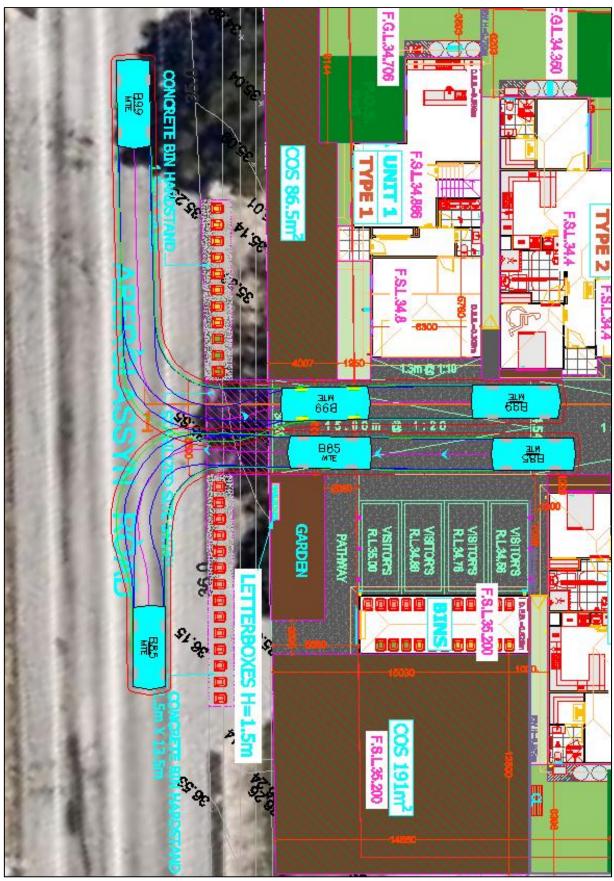


AUSTRALIAN STANDARD 85<sup>TH</sup> PERCENTILE SIZE VEHICLE (B85)

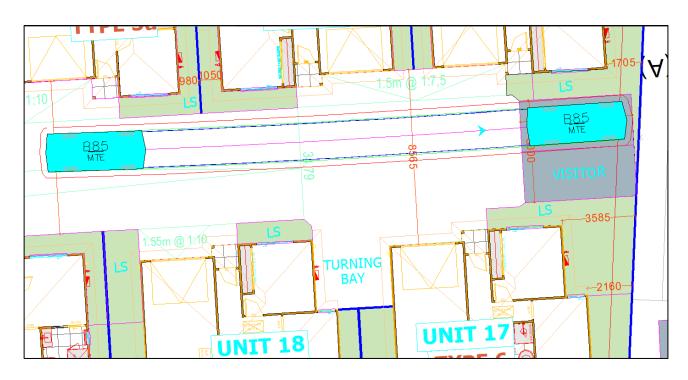


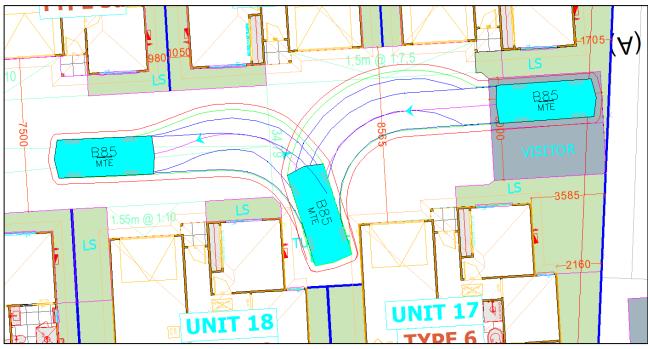
AUSTRALIAN STANDARD 99.8<sup>TH</sup> PERCENTILE SIZE VEHICLE (B99)

Blue – Tyre Path Green – Vehicle Body Red – 300mm Clearance



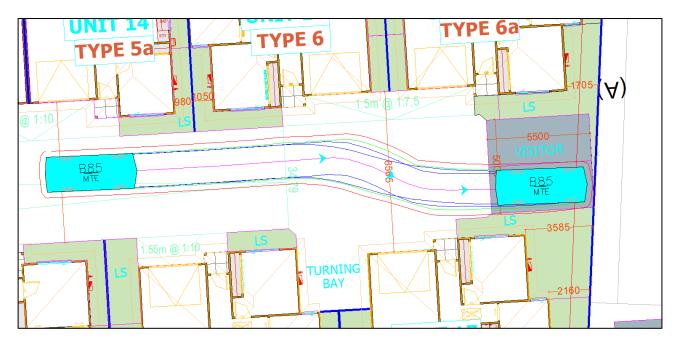
Circulation – Two-way passing at site boundary
B99 entering / B85 exiting
SUCCESSFUL

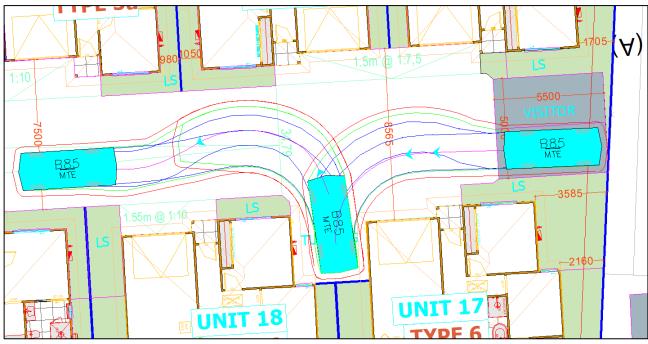




B85 Entry / Exit from Visitor Space 5

1 Manoeuvre FORWARD IN / 2 Manoeuvres REVERSE OUT
SUCCESSFUL





B85 Entry / Exit from Visitor Space 6

1 Manoeuvre FORWARD IN / 2 Manoeuvres REVERSE OUT
SUCCESSFUL



