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# Buildings

## 10-12/91 Gardiner Street, Rutherford

# Stormwater Management Plan

JCPMB Pty Ltd

Revision: 5  
Version Date: 17 February 2025  
GCA Ref: 23263C

LAND DEVELOPMENT • BUILDINGS • INFRASTRUCTURE  
CIVIL, STRUCTURAL & ENVIRONMENTAL ENGINEERING,  
WATER & WASTEWATER, BUILDING DESIGN & PROJECT ADVISORY

Revision	Description	Author		Review		Approved	
1	Initial Issue	SK	06.03.24	SH		SH	
2	Revised Hydrology	SK	11.03.24	SH	11.03.24	SH	11.03.24
3	General Amendments	AM	10.05.24	IH	10.05.24	IH	10.05.24
4	Combined Modelling	SK	18.11.24	SH	29.11.24	IH	29.11.24
5	General Amendments	SK	17.02.25	SH	17.02.25	SH	17.02.25

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## List of Acronyms

MCC	Maitland City Council
MOES	Manual of Engineering Standards
`OSD	On Site Detention
PSD	Permissible Site Discharge
SSR	Site Storage Requirement
SQUIDS	Stormwater Quality Improvement Devices

# 1 Background

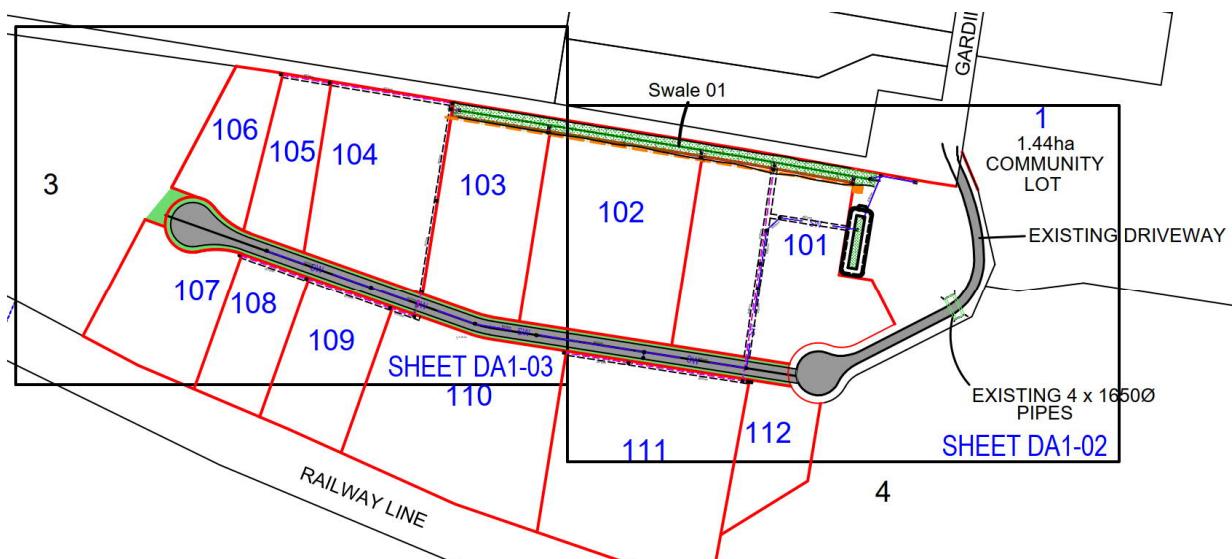
This report is to support a Development Application to Maitland City Council for a proposed light industrial development at 10/91-12/91 Gardiner Street, Rutherford.

## 1.1 Site

The site comprises Lots 6 and 7 on DP 271474, which were known as Lots 105 and 106 in the parent Subdivision Works Certificate plan, refer Figure 1.

The lots discharge to an interallotment drainage line that conveys flow to Swale 01, which has been designed to treat runoff to meet Council's water quality targets. Accordingly, water quality controls are not required for the subject development.

This report confirms that the proposed-on site detention systems ensure that the post development volume rate of flow is less than the predevelopment case for the 20%, 5% and 1% Annual Exceedance Probability events.



**Figure 1: Subdivision Water Management Plan**

## 1.2 Proposed Development

The proposed development comprises the construction of 8 industrial buildings with associated curtilage including car parking, concrete pavement, and landscaping. The total site area including the proposed access driveway is 10,715m<sup>2</sup>.

## 1.3 Stormwater Management Strategy

To minimise the on ground on site detention, roof water is to be detained in 10,000 Litre above ground tanks with 20mm discharge orifices (30mm for the twin tanks at Building H). Tanks are to be located generally at the sides of the proposed buildings.

Runoff from the rooves is to be collected in gutters and directed to the tanks. 150mm diameter overflows are to be connected at the tops of the tanks and directed to the on-site drainage system. Low flow outlets, comprising 100mm diameter pipes and orifices (as indicated on the plans) are to be located 100mm above the invert of the tanks and connected to the high-level overflows at ground level.

Runoff from ground level catchments generally cumulates to surface detention systems, whose outlets are controlled by orifices on the downstream faces of the control pits as indicated on Drawing C01. The differences between the incoming runoff and discharge are to be stored in the designed pavement depressions.

Generally, each of the surface detention systems overflows when full to the main driveway, where the overflow is collected and further detained in the above ground OSD system 7 at Pit 70.

Further detailed calculations are provided in Section 2.

## 2 On Site Detention

### 2.1 Requirements

Maitland City Council provides guidance for onsite detention systems in its Manual of Engineering Standards 2014.

For sites larger than 3000m<sup>2</sup>, detailed modelling of the stormwater system using time area hydrograph methodology is required to demonstrate the pre development flow rates are matched (or improved on) in the 20%, 5% and 1% AEP events.

### 2.2 Methodology

ARR2019 provides the current methodology for determining design rainfall events as ensembles of storms. It uses up-to-date rainfall data and provides an area- based procedure relative to the location of the subject site. Note also that ARR2019 discourages the use of the term “Average Recurrence Interval (ARI)”.

The DRAINS software package published by Watercom Pty Ltd is a time area hydrograph model that translates rainfall hyetographs into runoff hydrographs over sub catchments and subsequently adds the resulting hydrographs together to quantify design rates of flow and runoff volumes. DRAINS has inbuilt procedures for accommodating ARR2016 rainfall patterns and takes a suite of rainfall events (of various duration) for the same exceedance probability and runs them through the catchment - node model to determine maximum rate of flow at any point. Note that the maximum rate of flow at one point in the system does not necessarily originate from the same duration storm as at another point in the system.

The stie's DRAINS model was built with both predevelopment and post development outlet links so that the respective outflow rates could be compared directly.

Refer Appendix B for DRAINS Model layout, data and results. Results are summarised as Section 2.3 Below:

### 2.3 Results

**Table 2-1 Designed OSD Volume Summary.**

Description	1% AEP WATER LEVEL (m AHD)	1% AEP DETENTION VOLUME (m <sup>3</sup> )	1% AEP FLOW RATE (m <sup>3</sup> s <sup>-1</sup> )
SURFACE BASIN 1	23.27	11.7	0.036
SURFACE BASIN 2	23.21	16.3	0.044
SURFACE BASIN 3	22.46	9.2	0.035
SURFACE BASIN 4	23.46	3.7	0.030
SURFACE BASIN 5	22.52	19.6	0.093
SURFACE BASIN 6	23.11	26.8	0.057
SURFACE BASIN 7	22.44	50.2	0.265
COMINED ROOF WATER TANKS		86.6	
TOTAL DETENTION		224.1	

**Table 2-2 Total Flow Rates**

Event	Predevelopment Discharge Rate ( $\text{m}^3\text{s}^{-1}$ )	Post Development Discharge Rate ( $\text{m}^3\text{s}^{-1}$ )
20% AEP	0.107	0.101
5% AEP	0.233	0.200
1% AEP	0.412	0.406

## 3 Stormwater Quality

### 3.1 Requirements

Water quality targets are given in Section 8.2 of the Maitland City Council's Manual of Engineering Standards and are reflective of best practice for the region. It should be noted that the targets are benchmarked against what is likely to be generated by the proposed development. Council's targets are indicated in Table 3-1:

**Table 3-1 Post Construction Stormwater Management Targets**

Pollutant	Retention Criteria
Suspended Solids	80% of average annual load
Total Phosphorous	45% of average annual load
Total Nitrogen	45% of average annual load
Gross Pollutants > 5mm	70% of average annual load

### 3.2 Methodology

The water quality controls for this development were provided in Swale 01 at the subdivision stage, as indicated on Figure 1. The proposed development complies with Council's targets for water quality.

## 4 Conclusion

The design stormwater system comprises:

- Collection of roof water and associated on site detention provided in above ground rainwater tanks with 10,000L and 15,000L capacity for Buildings G and H respectively. Tanks are to have 35mm (Building G) and 45mm (Building H) orifices installed 100mm above their inverts.
- Collection of surface water at a centralised pits and shaping of the pavement to contain a total of 137.5m<sup>3</sup> storage and safe overflow path to the rear of the site.

The development proposal complies with Maitland City Council's requirements for stormwater management.

## Appendix A

### DEVELOPMENT PLANS

# PROPOSED UNIT DEVELOPMENT

10/91-12/91 GARDINER STREET, RUTHERFORD  
DEVELOPMENT APPLICATION

JCPMB PTY LTD

MAITLAND CITY COUNCIL  
FEBRUARY 2025



DRAWING SCHEDULE			23263
DWG No.	SHEET TITLE	REV	
C00	COVER SHEET	5	
C01	CIVIL WORKS PLAN	5	
C02	SEDIMENT AND POLLUTION CONTROL PIT	3	
C03	RAINWATER TANK DETAILS	3	
C05	EARTHWORK PLAN	5	



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## NOTE

ALL EXISTING UNDERGROUND SERVICES MUST BE LOCATED AND EXPOSED PRIOR TO EARTHWORKS COMMENCING AND IT IS THE RESPONSIBILITY OF THOSE PERSONS USING THIS PLAN TO CONFIRM BOTH POSITION & LEVEL OF THESE UTILITIES IN CONJUNCTION WITH THE APPROPRIATE AUTHORITY.

## LEGEND

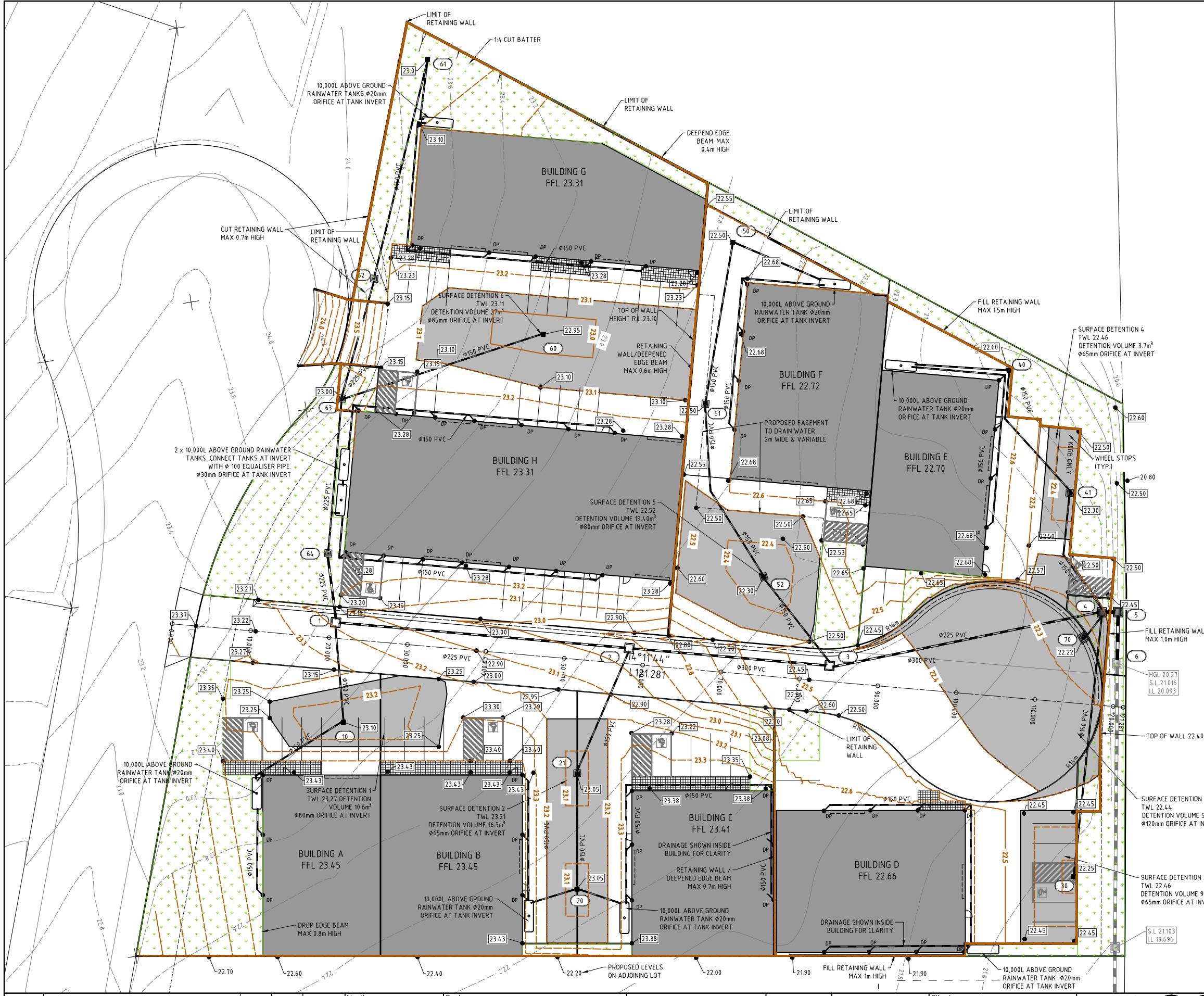
	PROPOSED BUILDING
	PROPOSED DETENTION BASIN
	PROPOSED LANDSCAPE

FFL	FINISHED FLOOR LEVEL
DP	DOWNSPIPE
PIT NUMBER	PIT NUMBER
FLOW	FLOW DIRECTION
KERB ONLY	KERB ONLY
PROPOSED PIPE	PROPOSED PIPE
EXISTING PIPE	EXISTING PIPE
OHP	EXISTING OVERHEAD POWER
S	EXISTING SEWER
12.00	DESIGN FINISH PAVEMENT LEVEL
12.00	EXISTING DESIGN FINISH PAVEMENT LEVEL
■	GRADED SURFACE INLET PIT (GSIP)
□	JUNCTION BOX (JB)
■	EXISTING GSIP

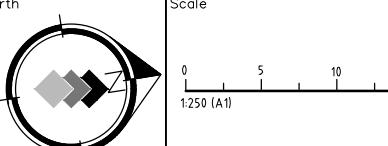
## DRAINAGE SCHEDULE

Pit No.	DESCRIPTION	LEVEL	
		SURFACE	PIT INVERT
1	900 x 900 JUNCTION	23.20	22.09
2	900 x 900 JUNCTION	23.93	21.91
3	900 x 900 JUNCTION	22.48	21.43
4	900 x 900 GSIP	22.22	20.26
5	900 x 900 GSIP	21.10	20.15
6	EXIST IAD PIT	21.01	20.09
10	450 x 450 PCP	23.10	22.60
20	450 x 450 PCP	23.05	22.55
21	450 x 450 PCP	23.05	22.48
30	450 x 450 PCP	22.25	21.75
40	450 x 450 SPCP	22.60	22.10
41	600 x 600 PCP	22.30	21.55
50	450 x 450 SPCP	22.50	22.00
51	600 x 600 SPCP	22.50	21.80
52	600 x 600 PCP	22.30	21.57
60	450 x 450 PCP	22.95	22.45
61	450 x 450 SPCP	23.00	22.50
62	600 x 600 SPCP	23.00	22.36
63	601 x 600 SPCP	23.00	22.19
64	900 x 900 GSIP	23.35	22.09
70	450 x 450 PCP	22.22	21.72

PRELIMINARY ISSUE  
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Amendment	Description	Drawn	App'd	Date
5	ARCHITECTURAL AMENDMENTS	SK	SH	17.02.25
4	ARCHITECTURAL AMENDMENTS	SK	SH	29.11.24
3	LAYOUT AMENDMENTS	AM	IH	10.05.24
2	REVISED HYDROLOGY	SK	SH	08.03.24
1	FIRST ISSUE	SK	SH	22.02.24



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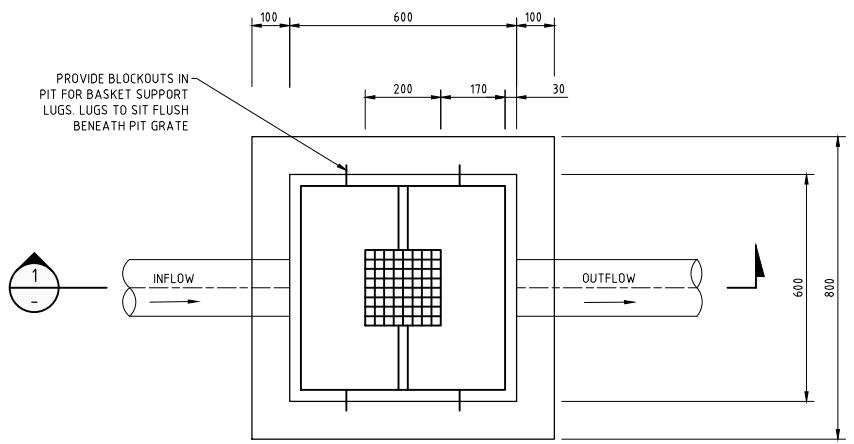
Designed  
S.K  
Scale  
1:250  
A1 SHEET  
Project Approval  
IAN HILL (B.E)  
Consulting Civil Engineer



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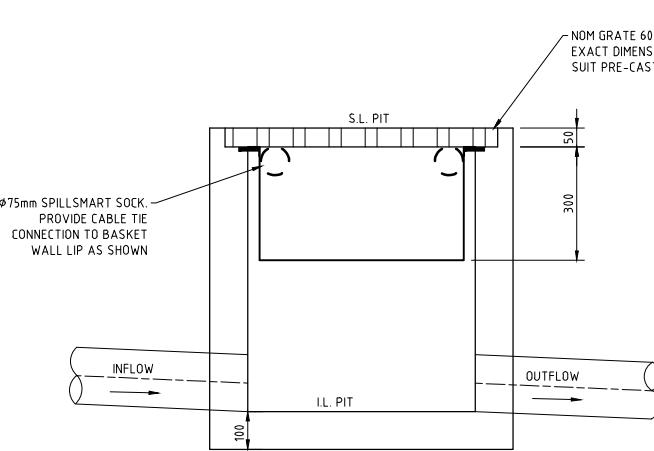
PROPOSED UNIT DEVELOPMENT  
10/91-12/91 GARDINER STREET  
RUTHERFORD  
CIVIL WORKS PLAN

Project No  
23263C  
Drawing No  
C01  
Revision  
5



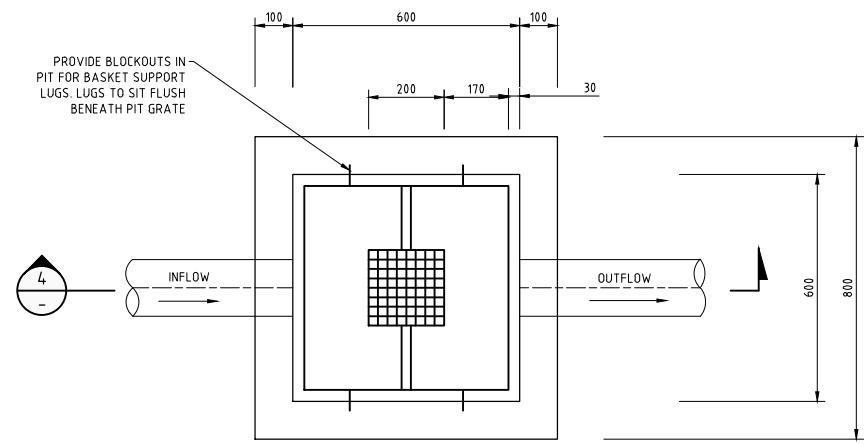
**POLLUTION PIT TYPICAL DETAIL (PCP)**

SCALE 1:10



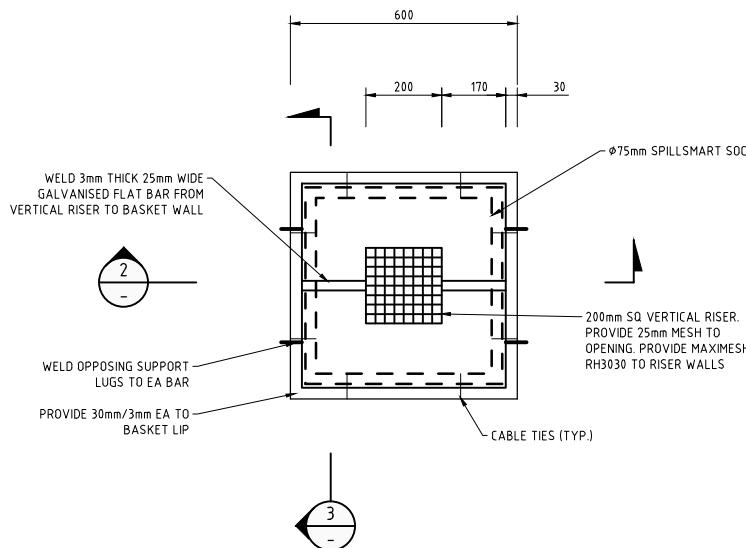
**TYPICAL SECTION THROUGH PCP - SECTION 1**

SCALE 1:10



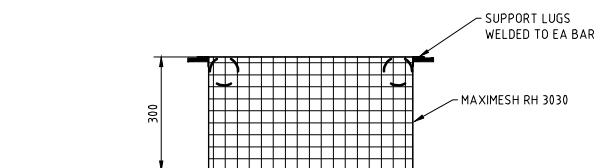
**SEDIMENT PIT TYPICAL DETAIL**

SCALE 1:10



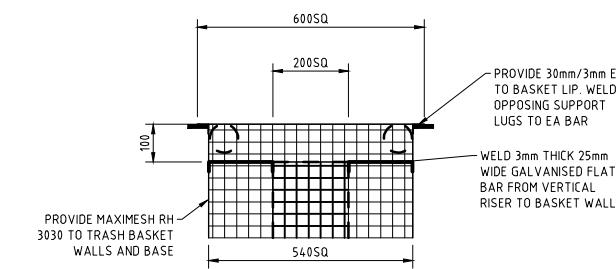
**PCP TRASH BASKET TYPICAL DETAIL**

SCALE 1:10



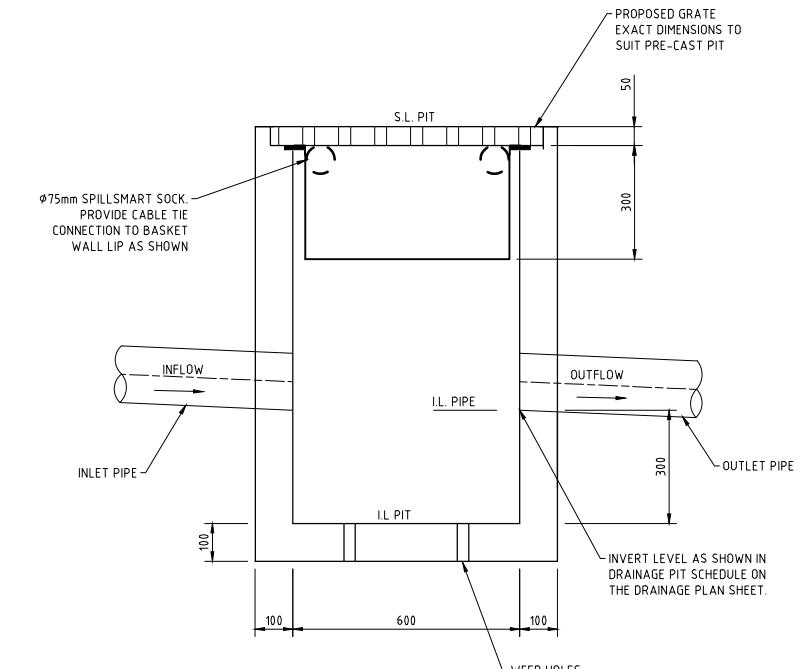
**SECTION 3**

SCALE 1:10



**SECTION 2**

SCALE 1:10

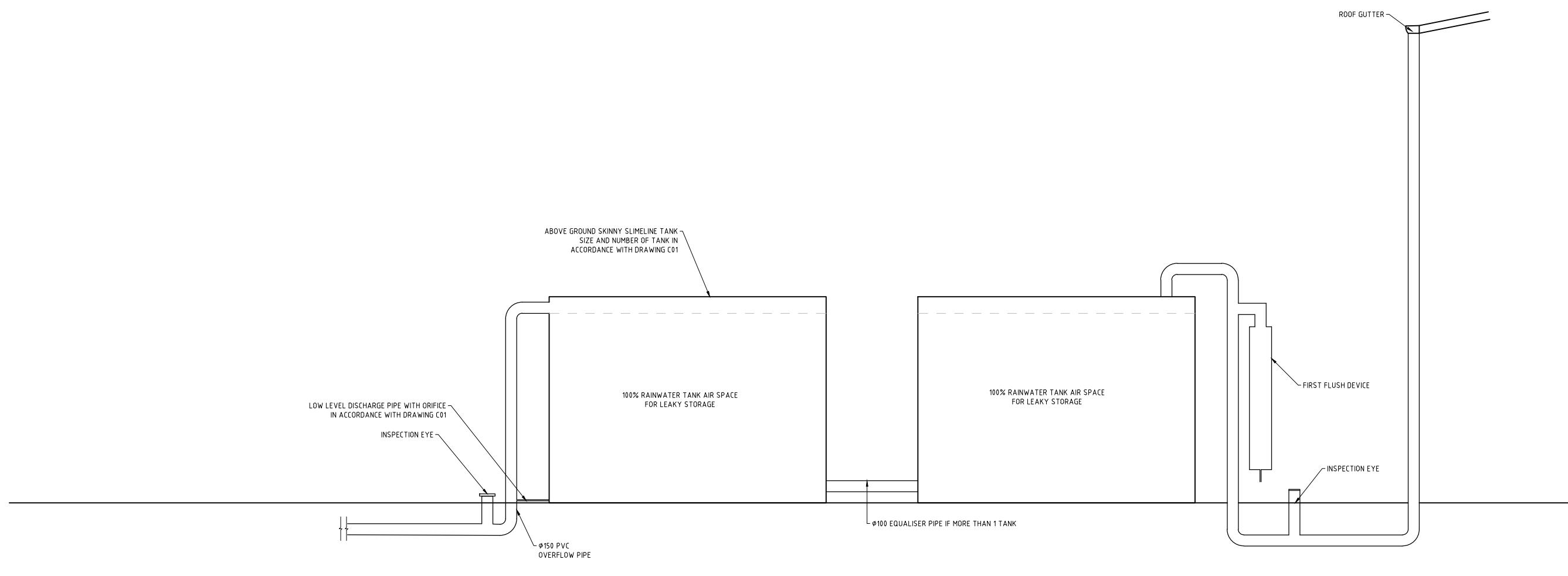


**SECTION THROUGH SEDIMENT PIT 4**

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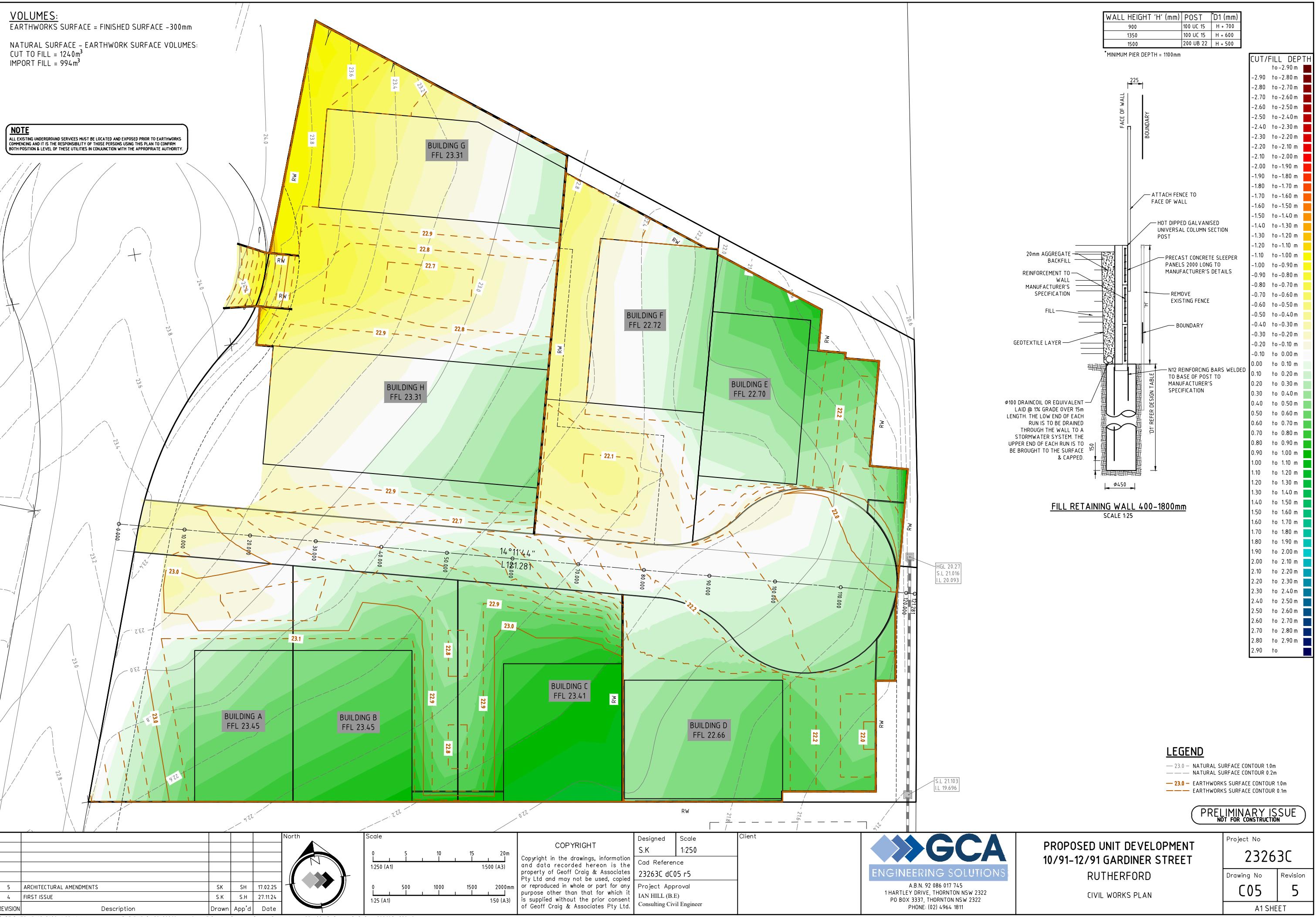
**PRELIMINARY ISSUE  
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Amendment	Description	Drawn	App'd	Date	North	Scale	COPYRIGHT	Designed	Cad Reference	Client	GCA	PROPOSED UNIT DEVELOPMENT	Project No
3	GENERAL REVISION	A.M	I.H	24.04.24		0 200 400 600 800mm 1:10 (A1) 1:20 (A3)	Copyright in the drawings, information and data recorded hereon is the property of Geoff Craig & Associates Pty Ltd and may not be used, copied or reproduced in whole or part for any purpose other than that for which it is supplied without the prior consent of Geoff Craig & Associates Pty Ltd.	S.K	23263C dC02 r3		Engineering Solutions	10/91-12/91 GARDINER STREET RUTHERFORD	23263C
2	REVISED HYDROLOGY	S.K	S.H	08.03.24		Scale	A1 SHEET	I.AN HILL (B.E)			A.B.N. 92 086 017 745 1 HARTLEY DRIVE, THORNTON NSW 2322 PO BOX 3337, THORNTON NSW 2322 PHONE: (02) 4964 1811	SEDIMENT AND POLLUTION CONTROL PIT DETAILS	
1	FIRST ISSUE	S.K	S.H	22.02.24		Project Approval		Consulting Civil Engineer				Drawing No	Revision
											C02	3	



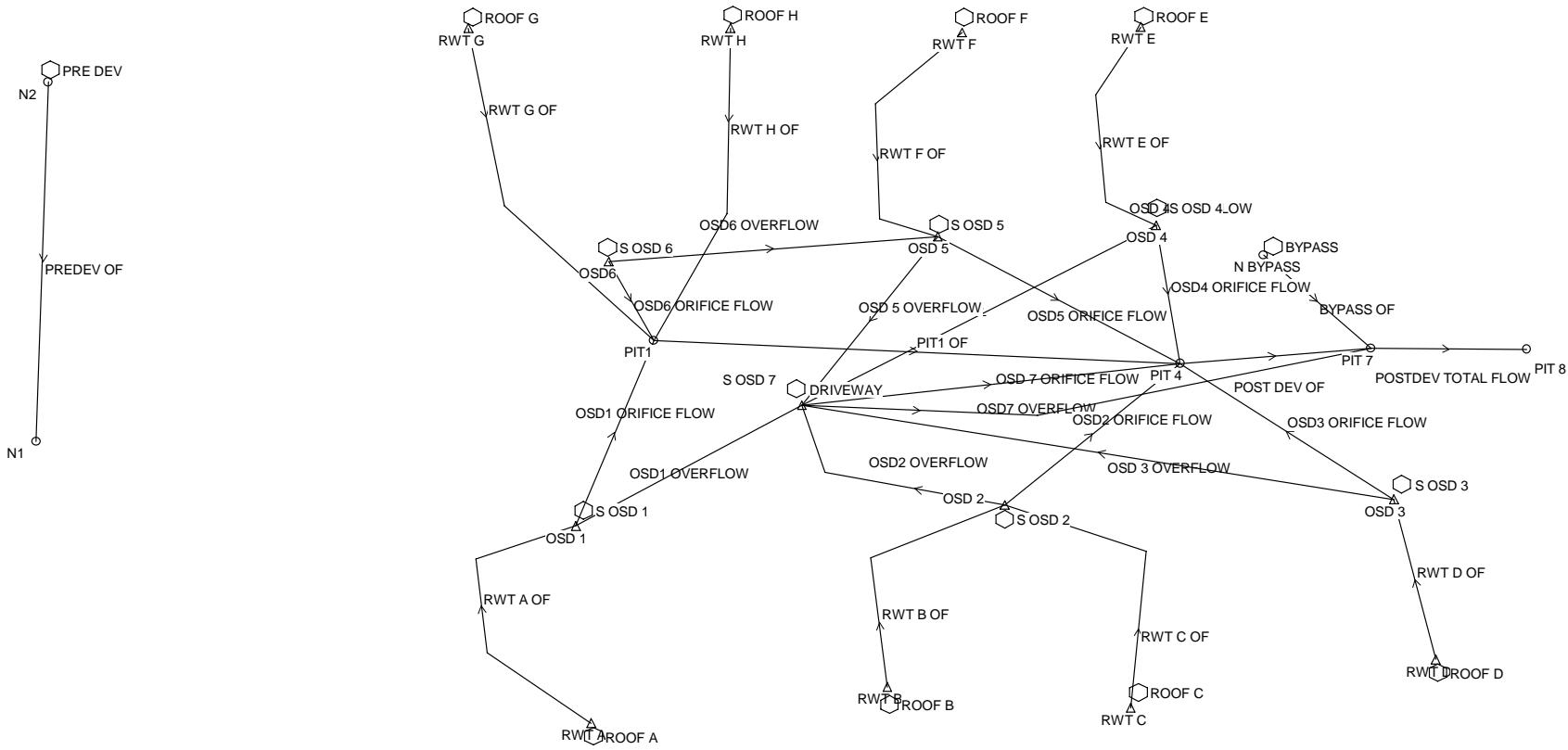
PRELIMINARY ISSUE  
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3	GENERAL REVISION		A.M	I.H	24.04.24									
2	REVISED HYDROLOGY		S.K	S.H	08.03.24									
1	FIRST ISSUE		S.K	S.H	22.02.24									
Amendment	Description		Drawn	App'd	Date									
North				Scale			COPRIGHT		Designed	Cad Reference				
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				1:500 (A1)	1:1000 (A3)				Scale	A1 SHEET				
									Project Approval	GCA	PROPOSED UNIT DEVELOPMENT			
									IAN HILL (B.E)	ENGINEERING SOLUTIONS	10/91-12/91 GARDINER STREET			
									Consulting Civil Engineer		RUTHERFORD			
											RAINWATER TANK DETAILS			
											Project No			
											23263C			
											Drawing No			
											C03			
											Revision			
											3			



## Appendix B

### DRAINS MODELLING





Name	From	To	Length (m)	U/S IL (m)	D/S IL (m)	Slope (%)	Type	Dia (mm)	I.D. (mm)	Rough	Pipe Is	No. Pipes	Chg From	At Chg	Chg (m)	Rl (m)	Chg (m)	RL (m)
<b>DETAILS OF SERVICES CROSSING PIPES</b>																		
Pipe	Chg (m)	Bottom Elev (m)	Height of S	Chg (m)	Bottom Elev (m)	Height of S	Chg (m)	Bottom Elev (m)	Height of S	etc								
<b>CHANNEL DETAILS</b>																		
Name	From	To	Type	Length (m)	U/S IL (m)	D/S IL (m)	Slope (%)	Base (m)	Width (1:?)	L.B. Slope (1:?)	R.B. Slope (1:?)	Manning n	Depth (m)	Roofed				
<b>OVERFLOW ROUTE DETAILS</b>																		
Name	From	To	Travel Time (min)	Spill Level (m)	Crest Length (m)	Weir Coeff. C	Cross Section	Safe Major (m)	Depth Storm (m)	Safe Minor (m)	Bed DxV (sq.m/sec)	Slope (%)	D/S Area Contributing %	id	U/S IL	D/S IL	Length (m)	
PREDEV OF	N2	N1	0.3				OVERFLOW	0.3	0.3	0.4	1	0	9	22.5	20	5		
RWT G OF	RWT G	PIT1	0.3	23.2			OVERFLOW	0.3	0.3	0.4	1	0	48	23.2	23	5		
RWT H OF	RWT H	PIT1	0.3	23.2			OVERFLOW	0.3	0.3	0.4	1	0	49	23.2	23.1	5		
OSD6 ORIFICE FLOW	OSD6	PIT1	2.8	22.45			OVERFLOW	0.3	0.3	0.4	1	0	369806	22.45	22.1	50		
OSD6 OVERFLOW	OSD6	OSD 5	0.6	23.1			OVERFLOW	0.3	0.3	0.4	1	0	286247	23.1	22.3	10		
RWT F OF	RWT F	OSD 5	0.3	22.6			OVERFLOW	0.3	0.3	0.4	1	0	54	22.6	22.5	5		
OSD 5 OVERFLOW	OSD 5	S OSD 7	0.1	22.5			channel between basins	0.3	0.3	0.6	1	0	674590	22.5	22.25	10		
OSD5 ORIFICE FLOW	OSD 5	PIT 4	0.1	21.73			channel between basins	0.3	0.3	0.6	1	0	286246	21.73	20.5	10		
RWT E OF	RWT E	OSD 4	0.3	22.6			OVERFLOW	0.3	0.3	0.4	1	0	74	22.6	22.5	5		
OSD 4 OVERFLOW	OSD 4	S OSD 7	0.1	22.45			channel between basins	0.3	0.3	0.6	5	0	1340440	22.45	20.5	5		
OSD4 ORIFICE FLOW	OSD 4	PIT 4	0.3	21.63			OVERFLOW	0.3	0.3	0.4	1	0	76	21.63	20.5	5		
POST DEV OF	PIT 4	PIT 7	0.1				OVERFLOW	0.3	0.3	0.4	1	0	139	20.5	20	2		
RWT A OF	RWT A	OSD 1	0.3	23			OVERFLOW	0.3	0.3	0.4	1	0	91	23	22.75	5		
OSD1 OVERFLOW	OSD 1	S OSD 7	0.1	23.25			channel between basins	0.3	0.3	0.6	1	0	650173	23.25	22.25	10		
OSD1 ORIFICE FLOW	OSD 1	PIT1	0.6	22.6			OVERFLOW	0.3	0.3	0.4	1	0	369813	22.35	22.1	10		
RWT B OF	RWT B	OSD 2	0.3	23.1			OVERFLOW	0.3	0.3	0.4	1	0	110	23.1	23	5		
RWT C OF	RWT C	OSD 2	0.3	23.1			OVERFLOW	0.3	0.3	0.4	1	0	111	23.1	22.9	5		
OSD2 OVERFLOW	OSD 2	S OSD 7	0.3	23.2			channel between basins	0.3	0.3	0.6	1	0	586996	23.2	22.25	50		
OSD2 ORIFICE FLOW	OSD 2	PIT 4	0.6	22.4			OVERFLOW	0.3	0.3	0.4	1	0	286254	22.2	20.5	10		
RWT D OF	RWT D	OSD 3	0.3	22.6			OVERFLOW	0.3	0.3	0.4	1	0	234906	22.6	22.55	5		
POSTDEV TOTAL FLOW	PIT 7	PIT 8	0.1				OVERFLOW	0.3	0.3	0.4	1	0	1349630	20	19.5	2		
OSD7 OVERFLOW	S OSD 7	PIT 7	0.2	22.4			channel between basins	0.3	0.3	0.6	5	0	1349618	22.3	20	50		
OSD 7 ORIFICE FLOW	S OSD 7	PIT 4	0.3	21.5			OVERFLOW	0.3	0.3	0.4	1	0	286255	21.5	20.5	5		
OSD 3 OVERFLOW	OSD 3	S OSD 7	0.1	22.45			channel between basins	0.3	0.3	0.6	1	0	666254	22.45	22.25	10		
OSD3 ORIFICE FLOW	OSD 3	PIT 4	0.6	21.65			OVERFLOW	0.3	0.3	0.4	1	0	286256	21.85	20.5	10		
PIT1 OF	PIT1	PIT 4	2.3				OVERFLOW	0.3	0.3	0.4	1	0	323256	22.1	20.5	40		
BYPASS OF	N BYPASS	PIT 7	0.3				OVERFLOW	0.3	0.3	0.4	1	0	824544	21	20	5		

**PIPE COVER DETAILS**

Name	Type	Dia (mm)	Safe Cover Cover (m)
This model has no pipes with non-return valves			





