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	DESIGN:	M.M				CATHER	INE S	STRI	INF EE		
t or a Third Party where the information is incorrect, incomplete, inaccurate, out-of-date or unreasonable; ed from a Third Party; or inaccurate;	DRAWN:	M.M		HOOVE	ER GRC	OUP	60 CATH	ERIN	IE S	TR	
an where recommended by AL Civil Design; mmendation of AL Civil Design; oximations or estimates made or referred to by AL Civil Design in this plan. distributed, or reproduced by any process unless this note is clearly displayed on the plan. should be verified by checking against the bar scale. survey and have been determined by plan dimensions only and not by field survey. accords of relevant authorities where available. Prior to any demolition, excavation or construction on the urther underground services and detailed locations of all service	CHECKED:	M.M			DRAWING TITLE						
	DATE:	20.11.2021	PLAN:	LONGITUDI							
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	GENERAL NOTES								
	DRAWINGS ARE CONCEPTUAL ONLY AN DEVELOPMENT APPLICATION TO COUNT THE FUTURE DETAILED DESIGN WORK.	ID HAVE BEEN PREPARED TO SUPPORT A CIL. DETAILS MAY CHANGE AS PART OF							
	DRAWINGS ARE TO BE READ IN CONJUN HOOVER GROUP	NCTION WITH ARCHITECTURAL PLANS BY							
т	 ARCHITECTURAL BASE ADOPTED FROM DESIGN BY HOOVER GROUP SURVEY INFORMATION ADOPTED FROM SURVEY BY DAVID CANT SURVEYORS. 								
ER	 ALL LEVELS ARE APPROXIMATE ONLY, (ALL LEVELS ARE FINISHED PAVEMENT (CONFIRM ON SITE. OR LAWN LEVELS.							
	EXISTING LEVELS SHOWN eg. 4.79E NEV	V LEVELS SHOWN eg. *20.84.							
WER	• THE EXISTING DEVELOPMENT IS VACAN	JT.							
ER PIT	THE PROPOSED DEVELOPMENT CONSIS UNITS AND ASSOCIATED DRIVEWAY AN	STS OF THE CONSTRUCTION OF THREE D LANDSCAPING.							
FALL	SITE ANALYSIS								
	THE SITE CONSISTS SLIGHT GRADE, SLISITE.								
	• A TYPICAL GRADE WOULD BE IN THE OF FROM 6.08 TO 5.67 AHD.	RDER OF 1.5% WITH LEVELS KANGING							
	STORMWATER MANAGEMENT								
	PLANS, HWC REQUIREMENTS AND PRO RECOMMENDATIONS.	PRIETARY MANUFACTURER'S							
	 RUNOFF FROM THE ROOF AREAS FOR T COLLECTED AND DIRECTED TO RAINWA PLAN. LOCATION OF TANKS & TANK TYF 	THE NEW DWELLINGS IS TO BE ATER REUSE TANKS AS SHOWN ON THE PES CAN BE VARIED AT OWNER'S							
	DISCRETION. • ROOF WATER IS TO PASS THROUGH A S	SUITABLE FIRST FLUSH BYPASS DEVICE							
	 PRIOR TO ENTERING THE TANK. HARVESTED RAINWATER IS TO BE REUSE 	SED FOR EXTERNAL IRRIGATION, TOILET							
	 TANK OVERFLOW IS TO BE CONVEYED DRAINAGE NETWORK TO THE DETENTIC 	THROUGH THE INTERNAL STORMWATER ON TANK AS SHOWN ON THE PLAN.							
	 ALL STORMWATER PIPES, INCLUDING D OVERFLOW PIPES, TO BE 100Ø UPVC S 	OWNPIPES AND RAINWATER TANK TORMWATER GRADE, U.N.O. JOINTED &							
	 INSTALLED TO MANUFACTURER'S RECO ALL DOWNPIPES TO BE FITTER WITH LE 	MMENDATIONS AF EATER™ RAIN HEAD TO PROVIDE							
	 UPVC PIPES TO CONFROM TO AS 1260. 								
	OVERFLOWS FROM THE DETENTION TA NOMINATED DISCHARGE POINT AS SHO MINIMUM COVER TO STORMWATER PIP	NK IS TO DISCHARGE TO THE WN ON THE PLAN.							
	TRAFFICABLE AREAS - 45	50mm							
	PIPES TO BE CONCRETE ENCASED IF M OBTAINED IN TRAFFICABLE AREAS, REF ALTERNATIVELY USED USE UPVC SEWE BUILDINGS.	INIMUM COVERS CANNOT BE ER TO CLAUSE 3.8 AS 3500.3 ER GRADE PIPES UNDER ROADS AND							
	STORMWATER								
	SITE AREA IMPERVIOUS AREA (ROOF)	≈ 675m ² ≈ 401.5m ²							
	IMPERVIOUS AREA (DRIVEWAY) PRE-DEVELOPED AVG. SLOPE	≈ 43m ² = 1.5%							
	DEVELOPED AVG. SLOPE	= 1.0%							
	FOLLOWING CLAUSE 7.8.2 OF THE MAITLAN ENGINEERING STANDARDS THE REQUIRED	ND CITY COUNCIL - MANUAL OF D DETENTION FOR THE DEVELOPMENT IS:							
	RAINWATER TANK OSD VOLUME FOR THE DETENTION NOT PROVIDED FOR ADDITION	ROOF AREA = $675m^2 \times 7m^3/1000m^2 = 4.7m^3$. JAL CONCRETE AREA AS AREA IS LESS							
	THEN 50m ² . PROVIDE 5m ³ OF DETENTION, TO BE SPLIT 2 RAINWATER TANKS TO HAVE 2m ³ OF DET HAVE 1m ³ OF DETENTION IN ADDITION THE	BETWEEN THE 3 x RAINWATER TANKS. ENTION AND 1 RAINWATER TANK TO							
	PERMEABLE SITE DISCHARGE 675/1000 x 1	5 = 0.010m ³ /s.							
	ORIFICE CALCULATION BASED ON TOP WA	TER LEVEL (MAX WATER LEVEL IN							
	RAINWATER TANK) OF 8.18m RL AND BOTT	OM OSD LEVEL 5.57m RL							
	$A_0 = Q_0 / G_1 \sqrt{2} gH_0$ $A_0 = Q_0 / C_0 \sqrt{2} gH_0$ $Q_1 = 0.010 m^3 (s (PSD = 101/s))$								
	$HEAD = H_0 = 2.61m$ g = 9.81m/s								
	$C_{d} = 0.61$								
	THEREFORE: ORIFICE DIAMETER REQUIRE EACH RAINWATER TANK. SEE DETAIL.	D IS 54mm. FITTED AS BLEED VALVE TO							
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ATER I	<u>·</u> PLAN								
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